ADHESIVE ANCHORS

PE1000+®

Epoxy Injection Adhesive Anchoring System

GENERAL INFORMATION

PE1000+®

Epoxy Injection Adhesive Anchoring System

PRODUCT DESCRIPTION

The PE1000+ is a two-component, high strength adhesive anchoring system. The system includes injection adhesive in plastic cartridges, mixing nozzles, dispensing tools and hole cleaning equipment. The PE1000+ is designed for bonding threaded rod and reinforcing bar hardware into drilled holes in concrete and solid masonry base materials.

GENERAL APPLICATIONS AND USES

- Bonding threaded rod and reinforcing bar into hardened concrete and grouted CMU
- Evaluated for use in dry and water-saturated concrete including water-filled holes
- Cracked and uncracked concrete
- Seismic and wind loading (see ESR-2583)
- Hammer-drill and diamond core drilled holes
- Can be installed in a wide range of base material temperatures

FEATURES AND BENEFITS

- + Designed for use with threaded rod and reinforcing bar hardware elements
- + Consistent performance in low and high strength concrete (2,500 to 8,500 psi)
- + Evaluated and recognized for freeze/thaw performance
- + Evaluated and recognized for long term and short term loading (see performance tables for applicable temperature ranges)
- + Evaluated and recognized for variable embedments (see installation specifications)
- + Cartridge design allows for multiple uses using extra mixing nozzles
- + Mixing nozzles proportion adhesive and provide simple delivery method into drilled holes
- + Easy dispensing reduces applicator fatigue

APPROVALS AND LISTINGS

- International Code Council, Evaluation Service (ICC-ES) ESR-2583
- Code compliant with the 2015 IBC, 2015 IRC, 2012 IBC, 2012 IRC, 2009 IBC, 2009 IRC, 2006 IBC, and 2006 IRC
- Tested in accordance with ACI 355.4 and AC308 for use in structural concrete according to ACI 318 Appendix D (Strength Design)
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading
- Compliant with NSF/ANSI Standard 61 for drinking water system components health effects; minimum requirements for materials in contact with potable waterand water treatment
- Conforms to requirements of ASTM C 881, Types I, II, IV and V, Grade 3, Classes B & C (also meets type III except for elongation)
- Department of Transportation listings see www.powers.com or contact transportation agency

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00 - Concrete Anchors, 04 05 19.16 - Masonry Anchors and 05 05 19 - Post-Installed Concrete Anchors. Adhesive anchoring system shall be PE1000+ as supplied by Powers Fasteners, Inc., Brewster, NY. Anchors shall be installed in accordance with published instructions and requirements of the Authority Having Jurisdiction.

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PE1000+ DUAL CARTRIDGE AND MIXING NOZZLE

PACKAGING

Dual (side-by-side) Cartridge

- 13 fl. oz. (385 ml)
- 20 fl. oz. (585ml)
- 47 fl. oz. (1400ml)

STORAGE LIFE & CONDITIONS

Two years in a dry, dark environment with temperature ranging from 41°F to 95°F(5°C to 35°C)

ANCHOR SIZE RANGE (TYP.)

- 3/8" to 1-1/4" diameter threaded rod
- No. 3 to No. 10 reinforcing bar (rebar)

SUITABLE BASE MATERIALS

- Normal-weight concrete
- Light-weight concrete
- Grouted concrete masonry

PERMISSIBLE INSTALLATION CONDITIONS

- Dry concrete
- Water-saturated concrete (wet)
- Water-filled holes (flooded)



This Product Available In



Powers Design Assist[®] Real Time Anchor Design Software www.powersdesignassist.com

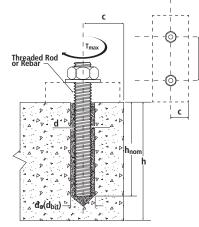
REFERENCE DATA (ASD)

Installation Table for PE1000+ (Solid Concrete Base Materials)

| Dimension/Prop | erty | Notation | Units | | | | N | ominal A | nchor Siz | e. | | | |
|---|-----------------|------------------------------------|--|----------------|-------------------|-------------|-------------------------|-----------------|-----------------|-----------------|-----------------|-----------------|---------------|
| Threaded Ro | d | - | - | 3/8" | 1/2" | - | 5/8" | 3/4" | 7/8" | 1" | - | 1-1/4" | - |
| Reinforcing B | Reinforcing Bar | | - | #3 | - | #4 | #5 | #6 | #7 | #8 | #9 | - | #10 |
| Nominal anchor diameter | | d | in. (mm) | 0.375 (9.5) | 0.5 (12 | | 0.625 (15.9) | 0.750 (19.1) | 0.875 (22.2) | 1.000 (25.4) | 1.125 (28.6) | 1.250 (31.8) | 1.25 (31.8 |
| Carbide drill bit nominal size | | d _o [d _{bit}] | in. | 7/16 ANSI | 9/16 ANSI | 5/8 Ansi | 11/16 or 3/4 ANSI | 7/8 ANSI | 1 ANSI | 1-1/8 ANSI | 1-3/8 ANSI | 1-3/8 ANSI | 1-1/ ANS |
| Diamond core bit nominal si | ze | d _o [d _{bit}] | in. | - | 5/ | 8 | 3/4 | 7/8 | 1 | 1-1/8 | - | - | - |
| Minimum nominal embedme | ent | h _{nom} | in. (mm) | 2-3/8 (61) | 2-3 (7 | | 3-1/8 (79) | 3-1/2 (89) | 3-1/2 (89) | 4 (102) | 4-1/2 (114) | 5 (127) | 5 (127 |
| Minimum spacing distance | | Smin | in. (mm) | 1-7/8 (48) | 2- <i>*</i> (6 | | 3-1/8 (80) | 3-3/4 (95) | 4-3/8 (111) | 5 (127) | 5-5/8 (143) | 6-1/4 (159) | 6-1/ (159 |
| Minimum edge distance | | Cmin | in. (mm) | | 1-3/4 (45) | | | | | | 2-3/4 (70) | | |
| Maximum targual | For $c \ge 5d$ | т. | ftlbf. | 15 (20) | 33 (44) | | 60 (81) | 105 (142) | 125 (169) | 165 (223) | - | 280 (379) | - |
| Maximum torque ¹ | For c < 5d | Tinst | (N-m) | 7 (9) | 1 (2 | | 27 (36) | 47 (63) | 56 (75) | 74 (100) | - | 126 (170) | - |
| Movimum torqual? | For $c \ge 5d$ | т | ftlbf. | 10 (13) | 2 (3 | | 50 (67) | 90 (122) | 125 (169) | 165 (223) | - | 280 (379) | - |
| Maximum torque ^{1,2} | For c < 5d | T _{inst} | (N-m) | 5 (6) | 1 (1 | | 22 (29) | 40 (54) | 56 (75) | 74 (100) | - | 126 (170) | - |
| Effective cross sectional area of threaded rod | | A _{se} | in.² (mm²) | 0.078 (50) | 0.1 (9 | | 0.226 (146) | 0.335 (216) | 0.462 (298) | 0.606 (391) | - | 0.969 (625) | - |
| Effective cross sectional area of reinforcing bar | | Ase | in. ² (mm ²) | 0.110 (71) | 0.2 (12 | | 0.310 (200) | 0.440 (284) | 0.600 (387) | 0.790 (510) | 1.000 (645) | - | 1.27 (819 |

2. Applies to ASTM A36/F 1554 Grade 36 threaded rods.

Detail of Steel Hardware Elements used with Injection Adhesive System



Nomenclature

- d
- = Diameter of anchor = Diameter of drilled hole = Base material thickness dbit h
- The minimum value of h should be 1.5hnom or 3", whichever is greater. hnom = Minimum embedment depth

| Steel Description (General) | Steel Specification (ASTM) | Nominal Anchor Size (inch) | Minimum Yield Strength fy (ksi) | Minimum Ultimate Strength fu (ksi) |
|--------------------------------|---|---------------------------------------|--|---|
| | A 36 or F 1554, Grade 36 | | 36.0 | 58.0 |
| Carbon Rod | F 1554 Grade 55 | 3/8 through 1-1/4 | 55.0 | 75.0 |
| | A 193, Grade B7 or F 1554, Grade 105 | | 105.0 | 125.0 |
| Stainless Rod | F 593 | 3/8 through 5/8 | 65.0 | 100.0 |
| (Alloy 304 / 316) | Condition CW | 3/4 through 1-1/4 | 45.0 | 85.0 |
| Grade 60 Reinforcing Bar | A 615, or A 767, A 996 | 3/8 through 1-1/4 (#3 through #10) | 60.0 | 90.0 |
| Grade 40 Reinforcing Bar | A 615 | 3/8 through 3/4 (#3 through #6) | 40.0 | 60.0 |

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Allowable Load Capacities for PE1000+ Installed into Uncracked Normal-Weight Concrete with Threaded Rod and Reinforcing Bar (Based on Bond Strength/Concrete Capacity)^{1,2,3,4,5,6}



| | | | Minimum Concrete Co | mpressive Strength, (f'c) | | | | | | | |
|------------------------------|-----------------------------|------------------|---------------------|---------------------------|-----------|--|--|--|--|--|--|
| Nominal Rod/Rebar Size | Minimum Embedment | 3,000 psi | 4,000 psi | 5,000 psi | 6,000 psi | | | | | | |
| (in. or #) | Embedment Depth (in.) | Tension (lbs) | | | | | | | | | |
| | 2-3/8 | 1,195 | 1,235 | 1,270 | 1,300 | | | | | | |
| 3/8 or #3 | 3-1/2 | 1,760 | 1,825 | 1,875 | 1,915 | | | | | | |
| | 4-1/2 | 2,265 | 2,345 | 2,410 | 2,460 | | | | | | |
| | 2-3/4 | 1,770 | 1,835 | 1,885 | 1,925 | | | | | | |
| 1/2 or #4 | 4-3/8 | 2,820 | 2,915 | 2,995 | 3,065 | | | | | | |
| | 6 | 3,865 | 4,000 | 4,110 | 4,200 | | | | | | |
| | 3-1/8 | 2,420 | 2,505 | 2,575 | 2,630 | | | | | | |
| 5/8 or #5 | 5-1/4 | 4,145 | 4,290 | 4,405 | 4,505 | | | | | | |
| | 7-1/2 | 5,970 | 6,180 | 6,345 | 6,485 | | | | | | |
| | 3-1/2 | 2,870 | 2,970 | 3,050 | 3,120 | | | | | | |
| | 6-1/4 | 5,715 | 5,915 | 6,075 | 6,210 | | | | | | |
| | 9 | 8,560 | 8,860 | 9,100 | 9,300 | | | | | | |
| | 3-1/2 | 2,870 | 2,970 | 3,050 | 3,120 | | | | | | |
| 7/8 or #7 | 7 | 7,285 | 7,540 | 7,745 | 7,915 | | | | | | |
| | 10-1/2 | 11,700 | 12,110 | 12,440 | 12,715 | | | | | | |
| | 4 | 3,505 | 3,630 | 3,725 | 3,810 | | | | | | |
| 1 or #8 | 8 | 9,570 | 9,905 | 10,175 | 10,400 | | | | | | |
| | 12 | 15,635 | 16,185 | 16,625 | 16,990 | | | | | | |
| | 4-1/2 | 4,185 | 4,330 | 4,445 | 4,545 | | | | | | |
| 1-1/8 or #9 | 9 | 12,025 | 12,445 | 12,785 | 13,065 | | | | | | |
| | 13-1/2 | 19,865 | 20,560 | 21,120 | 21,585 | | | | | | |
| | 5 | 4,900 | 5,070 | 5,210 | 5,325 | | | | | | |
| 1-1/4 or #10 | 10 | 15,030 | 15,560 | 15,980 | 16,335 | | | | | | |
| | 15 | 25,165 | 26,045 | 26,755 | 27,345 | | | | | | |

1. Allowable load capacities listed are calculated using an applied safety factor of 4.0. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety or overhead.

2. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.

3. The tabulated load values are applicable to single anchors installed at critical edge and spacing distances and at the minimum member thickness.

4. The tabulated load values are for applicable for dry concrete. Holes must be drilled with a hammer drill and an ANSI carbide drill bit. Installations in wet concrete or in water-filled holes may require a reduction in capacity. Contact Powers Fasteners for more information concerning these installation conditions.

5. Adhesives experience reductions in capacity at elevated temperatures. See the in-service temperature chart for allowable load capacity reduction factors.

6. Allowable bond strength/concrete capacity must be checked against allowable steel strength in tension to determine the controlling allowable load.



| Allowable Load Capacities for PE1000+ Installed into Uncracked Normal-Weight Concrete |
|---|
| with Threaded Rod and Reinforcing Bar (Based on Steel Strength) ^{1,2,3,45,6} |



| Nominal | | | | S | teel Element | ts - Threade | d Rod and R | einforcing B | ar | | | | |
|--------------------------------|------------------------|----------------------|------------------------|----------------------|------------------------|-----------------------|------------------------|----------------------|------------------------|----------------------|---|------------------|--|
| Rod Diameter or Rebar | | | F1554, C | irade 55 | | ade B7 or rade 105 | F 593, (| CW (SS) | Grade 6 | 0 Rebar | Hear (kN) Tension (lbs (kN) Shear lbs (kN) ,690 2,185 1,125 7.5) (9.7) (5.0) ,005 3,890 2,005 13.4) (17.3) (8.9) ,695 6,075 3,130 20.9) (27.0) (13.9) ,760 8,745 4,505 | | |
| or Rebar Size (in. or #) | Tension lbs (kN) | Shear Ibs (kN) | Tension Ibs (kN) | Shear Ibs (kN) | Tension Ibs (kN) | Shear lbs (kN) | Tension lbs (kN) | Shear lbs (kN) | Tension Ibs (kN) | Shear lbs (kN) | lbs | lbs | |
| 3/8 or #3 | 2,115 (9.4) | 1,090 (4.8) | 2,735 (12.2) | 1,410 (6.3) | 4,555 (20.3) | 2,345 (10.4) | 3,645 (16.2) | 1,880 (8.4) | 3,280 (14.6) | 1,690 (7.5) | | | |
| 1/2 or #4 | 3,760 (16.7) | 1,935 (8.6) | 4,860 (21.6) | 2,505 (11.1) | 8,100 (36.0) | 4,170 (18.5) | 6,480 (28.8) | 3,340 (14.9) | 5,830 (25.9) | 3,005 (13.4) | | | |
| 5/8 or #5 | 5,870 (26.1) | 3,025 (13.5) | 7,595 (33.8) | 3,910 (17.4) | 12,655 (56.3) | 6,520 (29.0) | 10,125 (45.0) | 5,215 (23.2) | 9,110 (40.5) | 4,695 (20.9) | | | |
| 3/4 or #6 | 8,455 (37.6) | 4,355 (19.4) | 10,935 (48.6) | 5,635 (25.1) | 18,225 (81.1) | 9,390 (41.8) | 12,390 (55.1) | 6,385 (28.4) | 13,120 (58.4) | 6,760 (30.1) | | | |
| 7/8 or #7 | 11,510 (51.2) | 5,930 (26.4) | 14,885 (66.2) | 7,665 (34.1) | 24,805 (110.3) | 12,780 (56.8) | 16,865 (75.0) | 8,690 (38.7) | 17,860 (79.4) | 9,200 (40.9) | 11,905 (53.0) | 6,135 (27.3) | |
| 1 or #8 | 15,035 (66.9) | 7,745 (34.5) | 19,440 (86.5) | 10,015 (44.5) | 32,400 (144.1) | 16,690 (74.2) | 22,030 (98.0) | 11,350 (50.5) | 23,325 (103.8) | 12,015 (53.4) | 15,550 (69.2) | 8,010 (35.6) | |
| #9 | | | | | | | | | 29,680 (132.0) | 15,290 (68.0) | 19,785 (88.0) | 10,195 (45.3) | |
| 1-1/4 | 23,490 (104.5) | 12,100 (53.8) | 30,375 (135.1) | 15,645 (69.6) | 50,620 (225.2) | 26,080 (116.0) | 34,425 (153.1) | 17,735 (78.9) | | | | | |
| #10 | | | | | | | | | 37,625 (167.4) | 19,380 (86.2) | 25,080 (111.6) | 12,920 (57.5) | |

1. AISC defined steel strength (ASD): Tensile = $0.33 \bullet F_u \bullet A_{nom}$, Shear = $0.17 \bullet F_u \bullet A_{nom}$

2. Allowable load capacities listed are calculated for the steel element type. Consideration of applying additional safety factors may be necessary depending on the application, such as life safety or overhead.

3. The tabulated load values are applicable to single anchors at critical edge and spacing distances and at the minimum member thickness.

4. The tabulated load values are for dry concrete. Holes must be drilled with a hammer drill and an ANSI carbide drill bit. Installation in wet concrete or installations in water-filled holes may require a reduction in capacity. Contact Powers Fasteners for more information concerning these installation conditions.

5. Allowable shear capacity is controlled by steel strength for the given conditions.

6. Allowable bond strength/concrete capacity must be checked against allowable steel strength in tension to determine the controlling allowable load.

In-Service Temperature Chart for Allowable Load Capacities¹

| Base Materia | l Temperature | Bond Strength Reduction Factor for Temperature |
|--|--|--|
| °F | °C | Bond Strength Reduction Factor for Temperature |
| 41 | 5 | 1.00 |
| 50 | 10 | 1.00 |
| 68 | 20 | 1.00 |
| 75 | 14 | 1.00 |
| 104 | 40 | 0.85 |
| 110 | 43 | 0.82 |
| 122 | 50 | 0.76 |
| 140 | 60 | 0.69 |
| 1 Linear interpolation may be used to derive | reduction factors between those listed | |

REFERENCE DATA (ASD)



Grout-Filled Concrete Masonry Walls^{1,2}

ASD STORE

| Nominal | Drill | Minimum | Minimum | Minimum | Ultimat | e Load ³ | Allowat | le Load |
|------------------------------|-------------------------------------|-----------------------------------|---------------------------------|--------------------------------|-------------------------|-----------------------|-------------------------|-----------------------|
| Rod Diameter d. in. | Diameter d _{bit} in. | Embedment Depth in. (mm) | Edge Distance in. (mm) | End Distance in. (mm) | Tension Ibs. (kN) | Shear lbs. (kN) | Tension Ibs. (kN) | Shear Ibs. (kN) |
| 3/8 | 7/16 | 3 (76.2) | 2-1/2 (63.5) | 2-1/2 (63.5) | 3,350 (14.9) | 2,100 (9.3) | 670 (2.9) | 420 (1.9) |
| 1/2 | 9/16 | 4 (101.6) | 3 (76.2) | 3 (76.2) | 4,575 (20.3) | 2,550 (11.3) | 915 (4.1) | 510 (2.3) |
| 5/8 | 11/16 | 5 (127.0) | 3-3/4 (95.3) | 4 (101.6) | 6,900 (30.7) | 5,275 (23.5) | 1,380 (6.1) | 1,055 (4.7) |

Ultimate Load Capacities for Threaded Rod Installed with PE1000+ into the Block Face of

1. Tabulated load values are for anchors installed in minimum 8" wide, minimum Grade N, Type II, lightweight, medium-weight or normal-weight concrete masonry units conforming to ASTM C 90 that have reached a designated minimum compressive strength at the time of installation (f'm ≥1,500 psi). Mortar must be type N, S or M.

2. Anchor installations are limited to one per masonry cell. Shear loads may be applied in any direction.

3. The values listed are ultimate load capacities which should be reduced by a minimum safety factor of 5.0 or greater to determine the allowable working load. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety.

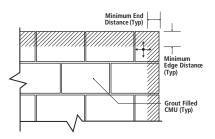
Load Capacities for Threaded Rod Installed with PE1000+ in the Top of Grout-Filled Concrete Masonry Walls^{1,2}

| Nominal | Drill | Minimum | Minimum | Minimum | Ultimat | e Load ³ | Allowat | ole Load |
|------------------------------|-------------------------------------|-----------------------------------|---------------------------------|--------------------------------|-------------------------|-----------------------|-------------------------|-----------------------|
| Rod Diameter d. in. | Diameter d _{bit} in. | Embedment Depth in. (mm) | Edge Distance in. (mm) | End Distance in. (mm) | Tension Ibs. (kN) | Shear lbs. (kN) | Tension Ibs. (kN) | Shear Ibs. (kN) |
| 1/2 | 9/16 | 6 (152.4) | 1-3/4 (44.5) | 3 (76.2) | 5,950 (26.4) | 1,450 (6.5) | 1,190 (5.3) | 290 (1.3) |
| 5/8 | 11/16 | 8 (203.2) | 1-3/4 (44.5) | 4 (101.6) | 9,450 (42.0) | 1,700 (7.5) | 1,890 (8.4) | 340 (1.4) |

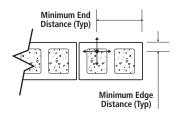
1. Tabulated load values are for anchors installed in a minimum Grade N, Type II, lightweight, medium-weight or normal-weight masonry units conforming to ASTM C 90 that have reached a designated minimum compressive strength at the time of installation (f'm ≥1,500 psi). Mortar must be type N, S or M.

2. Anchor installations are limited to one per masonry cell. Shear loads may be applied in any direction.

3. The values listed are ultimate load capacities which should be reduced by a minimum safety factor of 5.0 or greater to determine the allowable working load. Consideration of safety factors of 10 or higher may be necessary depending on the application, such as life safety.



Face Shell Permissible Anchor Locations (Un-hatched Area / Through Face Shell)



Top of Wall



STRENGTH DESIGN (SD)

Installation Specifications for Threaded Rod and Reinforcing Bar



Ultimate

Strength fu (ksi)

58.0

75.0

125.0

100.0

85.0

90.0

80.0

60.0

| Dimension/Proper | ty | Notation | Units | | | | N | ominal A | nchor Siz | ze | | | | | |
|--|-----------------|------------------------------------|---------------|----------------|---|-------------|-------------------------|-----------------|-----------------|-----------------------|-----------------|---|-----------------|--|--|
| Threaded Rod | | - | - | 3/8" | 1/2" | - | 5/8" | 3/4" | 7/8" | 1" | - | 1-1/4" | - | | |
| Reinforcing Bar | | - | - | #3 | - | #4 | #5 | #6 | #7 | #8 | #9 | #9 - 1.125 1.250 (28.6) (31.8) 1-3/8 1-3/8 ANSI ANSI - - 4-1/2 5 (114) (127) 22-1/2 25 (572) (635) 5-5/8 6-1/4 (143) 2-3/4 (70) 280 | | | |
| Nominal anchor diam | eter | d | in. (mm) | 0.375 (9.5) | 0.5 (12 | | 0.625 (15.9) | 0.750 (19.1) | 0.875 (22.2) | 1.000 (25.4) | 1.125 (28.6) | | 1.250 (31.8) | | |
| Carbide drill bit nominal size | | d _o [d _{bit}] | in. | 7/16 ANSI | 9/16 ANSI | 5/8 Ansi | 11/16 or 3/4 ANSI | 7/8 ANSI | 1 ANSI | 1-1/8 ANSI | | | 1-1/2 ANSI | | |
| Diamond core bit nomin | al size | d _o [d _{bit}] | in. | - | 5, | /8 | 3/4 | 7/8 | 1 | 1-1/8 | - | - | - | | |
| Minimum embedme | nt | h _{ef,min} | in. (mm) | 2-3/8 (61) | 2-3 (7 | | 3-1/8 (79) | 3-1/2 (89) | 3-1/2 (89) | 4 (102) | 4-1/2 (114) | | 5 (127) | | |
| Maximum embedme | nt⁴ | hef,max | in. (mm) | 4-1/2 (114) | 1 (25 | 0 54) | 12-1/2 (318) | 15 (381) | 17-1/2 (445) | 20 (508) | 22-1/2 (572) | | 25 (635) | | |
| Minimum concrete member | thickness | h _{min} | in. (mm) | | n _{ef} + 1-1/4 (h _{ef} + 30) | | | | - | h _{ef} + 2d₀ | | | | | |
| Minimum spacing dist | ance | S _{min} | in. (mm) | 1-7/8 (48) | 2- ⁻ (6 | 1/2 2) | 3-1/8 (80) | 3-3/4 (95) | 4-3/8 (111) | 5 (127) | | | 6-1/4 (159) | | |
| Minimum edge dista | nce | C _{min} | in. (mm) | | 1-3/4 (45) | | | | | | | | | | |
| Maximum torque ² | For $c \ge 5d$ | Tinst | ftlbf. | 15 (20) | 3 (4 | 3 4) | 60 (81) | 105 (142) | 125 (169) | 165 (223) | - | 280 (379) | - | | |
| Maximum torque | For c < 5d | l inst | (N-m) | 7 (9) | 1 (2 | 5 0) | 27 (36) | 47 (63) | 56 (75) | 74 (100) | - | 126 (170) | - | | |
| Maximum tarqua ² 3 | For $c \ge 5d$ | Tinst | ftlbf. | 10 (13) | 2 (3 | 5 3) | 50 (67) | 90 (122) | 125 (169) | 165 (223) | - | 280 (379) | - | | |
| Maximum torque ^{2,3} For c < 5d | | l inst | (N-m) | 5 (6) | 1 (1 | | 22 (29) | 40 (54) | 56 (75) | 74 (100) | - | 126 (170) | - | | |
| Effective cross sectional area of threaded rod | | A _{se} | in.² (mm²) | 0.078 (50) | 0.1 (9 | | 0.226 (146) | 0.335 (216) | 0.462 (298) | 0.606 (391) | - | 0.969 (625) | - | | |
| Effective cross sectional area of | reinforcing bar | A _{se} | in.² (mm²) | 0.110 (71) | 0.2 (12 | 200 29) | 0.310 (200) | 0.440 (284) | 0.600 (387) | 0.790 (510) | 1.000 (645) | - | 1.270 (819) | | |

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m. For pound-inch units: 1 mm = 0.03937 inch, 1 N-m = 0.7375 ft-lbf.

1. For use with the design provisions of ACI 318 Appendix D, ICC-ES AC308 Section 4.2 and ESR-2583

2. Torque may not be applied to the anchors until the full cure time of the adhesive has been achieved

3. These torque values apply to ASTM A36/F 1554 Grade 36 threaded rods

4. The maximum embedment is limited to 12 diameters for the horizontal and upwardly inclined installations and for installations in water-filled (flooded) holes with a carbide drill bit.

Minimum Minimum Steel Specification (ASTM) Steel Description (General) **Nominal Anchor** Yield Strength fy (ksi) Size (inch) A 36 or F 1554, 36.0 Grade 36 Carbon Rod F 1554 Grade 55 3/8 through 1-1/4 55.0 A 193, Grade B7 or 105.0 F 1554, Grade 105 3/8 through 5/8 65.0 Stainless Rod F 593 (Alloy 304 / 316) с Condition CW 3/4 through 1-1/4 45.0 A 615, or 60.0 A 767, A 996 3/8 through 1-1/4 Grade 60 h_{ef} (#3 through #10) Reinforcing Bar h A 706 60.0 Grade 40 3/8 through 3/4 A 615 40.0 Reinforcing Bar (#3 through #6)

Detail of Steel Hardware Elements used with Injection Adhesive System

Threaded Rod

'd₀(d_{bit}) –

Steel Tension and Shear Design for Threaded Rod in Normal Weight Concrete (For use with load combinations taken from ACI318 Section 9.2)

FASTENING INNOVATION

| | Destand Information | Completed. | 11 | | | Nominai | Nou Diamet | (incit) | | |
|--------------------------------|--|------------------|---|-----------------|------------------|-------------------|-------------------|-------------------|-------------------|--------------------|
| | Design Information | Symbol | Units | 3/8 | 1/2 | 5/8 | 3/4 | 7/8 | 1 | 1-1/4 |
| Threaded rod | nominal outside diameter | d | inch (mm) | 0.375 (9.5) | 0.500 (12.7) | 0.625 (15.9) | 0.750 (19.1) | 0.875 (22.2) | 1.000 (25.4) | 1.250 (31.8) |
| Threaded rod | effective cross-sectional area | Ase | inch ² (mm ²) | 0.0775 (50) | 0.1419 (92) | 0.2260 (146) | 0.3345 (216) | 0.4617 (298) | 0.6057 (391) | 0.9691 (625) |
| | Nominal strength as governed by | N _{sa} | lbf (kN) | 4,495 (20.0) | 8,230 (36.6) | 13,110 (58.3) | 19,400 (86.3) | 26,780 (119.1) | 35,130 (156.3) | 56,210 (250.0) |
| ASTM A 36 and | steel strength (for a single anchor) | V_{sa} | lbf (kN) | 2,695 (12.0) | 4,940 (22.0) | 7,860 (35.0) | 11,640 (51.8) | 16,070 (71.4) | 21,080 (93.8) | 33,725 (150.0) |
| ASTM F 1554 Grade 36 | Reduction factor for seismic shear | <i>Q</i> ℓV,seis | - | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Glude 50 | Strength reduction factor for tension ² | ϕ | - | | | | 0.75 | | | |
| | Strength reduction factor for shear ² | φ | - | | _ | | 0.65 | _ | | |
| | Nominal strength as governed by | Nsa | lbf (kN) | 5,810 (25.9) | 10,640 (47.3) | 16,950 (75.4) | 25,085 (111.6) | 34,625 (154.0) | 45,425 (202.0) | 72,680 (323.3) |
| ASTM F 1554 | steel strength(for a single anchor) | Vsa | lbf (kN) | 3,485 (15.5) | 6,385 (28.4) | 10,170 (45.2) | 15,050 (67.0) | 20,775 (92.4) | 27,255 (121.2) | 43,610 (194.0) |
| Grade 55 | Reduction factor for seismic shear | Ø∕v,seis | - | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| | Strength reduction factor for tension ² | φ | - | | | | 0.75 | | | |
| | Strength reduction factor for shear ² | ϕ | - | | | | 0.65 | | | |
| | Nominal strength as governed by | N _{sa} | lbf (kN) | 9,685 (43.1) | 17,735 (78.9) | 28,250 (125.7) | 41,810 (186.0) | 57,710 (256.7) | 75,710 (336.8) | 121,135 (538.8) |
| ASTM A 193 Grade B7 and | steel strength (for a single anchor) | Vsa | lbf (kN) | 5,815 (25.9) | 10,640 (7.3) | 16,950 (75.4) | 25,085 (111.6) | 34,625 (154.0) | 45,425 (202.1) | 72,680 (323.3) |
| ASTM F 1554 | Reduction factor for seismic shear | Ø∕v,seis | - | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| Grade 105 | Strength reduction factor for tension ² | ϕ | - | | | | 0.75 | | | |
| | Strength reduction factor for shear ² | φ | - | | | | 0.65 | | | |
| | Nominal strength as governed by | N _{sa} | lbf (kN) | 7,750 (34.5) | 14,190 (63.1) | 22,600 (100.5) | 28,430 (126.5) | 39,245 (174.6) | 51,485 (229.0) | 82,370 (366.4) |
| ASTM F 593 CW Stainless | steel strength (for a single anchor) | V_{sa} | lbf (kN) | 4,650 (20.7) | 8,515 (37.9) | 13,560 (60.3) | 17,060 (75.9) | 23,545 (104.7) | 30,890 (137.4) | 49,425 (219.8) |
| (Types 304 and 316) | Reduction factor for seismic shear | Ø∕V,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| | Strength reduction factor for tension ³ | ϕ | - | | | | 0.65 | | | |
| | Strength reduction factor for shear ³ | φ | - | | | | 0.60 | | | |
| ASTM A 193 | Nominal strength as governed by | N _{sa} | lbf (kN) | 4,420 (19.7) | 8,090 (36.0) | 12,880 (57.3) | 19,065 (84.8) | 26,315 (117.1) | 34,525 (153.6) | 55,240 (245.7) |
| Grade B8/B8M, Class 1 | steel strength (for a single anchor) ⁴ | V_{sa} | lbf (kN) | 2,650 (11.8) | 4,855 (21.6) | 7,730 (34.4) | 11,440 (50.9) | 15,790 (70.2) | 20,715 (92.1) | 33,145 (147.4) |
| Stainless (Types 304 | Reduction factor for seismic shear | Ø∕V,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| and 316) | Strength reduction factor for tension ² | φ | - | | | | 0.75 | | | |
| | Strength reduction factor for shear ² | φ | - | | | | 0.65 | | | |
| ASTM A 193 | Nominal strength as governed by | Nsa | lbf (kN) | 7,365 (32.8) | 13,480 (60.0) | 21,470 (95.5) | 31,775 (141.3) | 43,860 (195.1) | 57,545 (256.0) | 92,065 (409.5) |
| Grade B8/ B8M2, Class 2B | steel strength (for a single anchor) | V_{sa} | lbf (kN) | 4,420 (19.7) | 8,085 (36.0) | 12,880 (57.3) | 19,065 (84.8) | 26,315 (117.1) | 34,525 (153.6) | 55,240 (245.7) |
| Stainless | Reduction factor for seismic shear | Ø∕V,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| (Types 304 and 316) | Strength reduction factor for tension ² | φ | - | | | | 0.75 | | | |
| ., | Strength reduction factor for shear ² | ϕ | - | | | | 0.65 | | | |

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

1. Values provided for steel element material types are based on minimum specified strengths and calculated in accordance with ACI 318-11 Eq. (D-2) and Eq. (D-29) except where noted. Nuts and washers must be appropriate for the rod. Nuts must have specified proof load stresses equal to or greater than the minimum tensile strength of the specified threaded rod.

2. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC or ACI 318 Section 9.2 are used in accordance with ACI 318 D.4.3. If the load combinations of ACI 318 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to ductile steel elements.

3. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC or ACI 318 Section 9.2 are used in accordance with ACI 318-11 D.4.3. If the load

combinations of ACI 318 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to brittle steel elements

4. In accordance with ACI 318 D.5.1.2 and D.6.1.2 the calculated values for nominal tension and shear strength for ASTM A193 Grade B8/B8M Class 1 stainless steel threaded rods are based on limiting the specified tensile strength of the anchor steel to 1.9fy or 57,000 psi (393 MPa).

CODE LISTED ICC-ES ESR-2583

Nominal Rod Diameter¹ (inch)

7

TECH MANUAL – ADHESIVE ANCHORS © 2015 POWERS VOLUME 1 – 9/2015 – REV. G

Steel Tension and Shear Design for Reinforcing Bars in Normal Weight Concrete (For use with load combinations taken from ACI318 Section 9.2)



FASTENING INNOVATIONS

| • | | | | | - | | | | | | ABLES |
|---------------|--|-----------------------|----------------|------------------|------------------|-------------------|-------------------|-------------------|----------------------|---------------------------|------------------|
| | Dealers Information | Combal | Unite | | | Nomina | l Reinforcir | ng Bar Size | (Rebar) ¹ | | |
| | Design Information | Symbol | Units | No. 3 | No. 4 | No. 5 | No. 6 | No. 7 | No. 8 | No. 9 | No. 10 |
| Rebar nomir | al outside diameter | d | inch (mm) | 0.375 (9.5) | 0.500 (12.7) | 0.625 (15.9) | 0.750 (19.1) | 0.875 (22.2) | 1.000 (25.4) | 1.125 (28.7) | 1.250 (32.3 |
| Rebar effecti | ve cross-sectional area | A _{se} | inch² (mm²) | 0.110 (71.0) | 0.200 (129.0) | 0.310 (200.0) | 0.440 (283.9) | 0.600 (387.1) | 0.790 (509.7) | 1.000 (645.2) | 1.270 (819.4 |
| | Nominal strength as governed by | N _{sa} | lbf (kN) | 11,000 (48.9) | 20,000 (89.0) | 31,000 (137.9) | 44,000 (195.7) | 60,000 (266.9) | 79,000 (351.4) | 100,000 (444.8) | 127,00 (564.9 |
| ASTM A 615 | steel strength (for a single anchor) | Vsa | lbf (kN) | 6,600 (29.4) | 12,000 (53.4) | 18,600 (82.7) | 26,400 (117.4) | 36,000 (160.1) | 47,400 (210.8) | 60,000 (266.9) | 76,20 (338.9 |
| Grade 75 | Reduction factor for seismic shear | $\alpha_{\rm V,seis}$ | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| | Strength reduction factor for tension ³ | ϕ | - | | | | 0. | 65 | | | |
| | Strength reduction factor for shear ³ | ϕ | - | | | | 0. | 60 | | | |
| | Nominal strength as governed by steel strength (for a single anchor) | Nsa | lbf (kN) | 9,900 (44.0) | 18,000 (80.1) | 27,900 (124.1) | 39,600 (176.1) | 54,000 (240.2) | 71,100 (316.3) | 90,000 (400.3) | 114,30 (508.4 |
| ASTM A 615 | steel strength (for a single anchor) | Vsa | lbf (kN) | 5,940 (26.4) | 10,800 (48.0) | 16,740 (74.5) | 23,760 (105.7) | 32,400 (144.1) | 42,660 (189.8) | 54,000 (240.2) | 68,58 (305.) |
| Grade 60 | Reduction factor for seismic shear | <i>O</i> ℓV,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| | Strength reduction factor for tension ² | ϕ | - | 0.75 | | | | | | | |
| | Strength reduction factor for shear ² | ϕ | - | | ~ | ~ | 0. | 65 | ~ | - | - |
| | Nominal strength as governed by steel strength (for a single anchor) | N _{sa} | lbf (kN) | 8,800 (39.1) | 16,000 (71.2) | 24,800 (110.3) | 35,200 (156.6) | 48,000 (213.5) | 63,200 (281.1) | 80,000 (355.9) | 101,60 (452.0 |
| ASTM A 706 | steel strength (for a single anchor) | V _{sa} | lbf (kN) | 5,280 (23.5) | 9,600 (42.7) | 14,880 (66.2) | 21,120 (94.0) | 28,800 (128.1) | 37,920 (168.7) | 48,000 (213.5) | 60,96 (271.2 |
| Grade 60 | Reduction factor for seismic shear | <i>C</i> ℓv,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 | 0.80 |
| | Strength reduction factor for tension ² | ϕ | - | | | | 0. | 75 | | | |
| | Strength reduction factor for shear ² | ϕ | - | | | | 0. | 65 | | | |
| | Nominal strength as governed by | Nsa | lbf (kN) | 6,600 (29.4) | 12,000 (53.4) | 18,600 (82.7) | 26,400 (117.4) | In accor | dance with | ASTM A 61 | 5, Grade |
| ASTM A 615 | steel strength (for a single anchor) | Vsa | lbf (kN) | 3,960 (17.6) | 7,200 (32.0) | 11,160 (49.6) | 15,840 (70.5) | 40 bars | are furnishe | ed only in siz h No. 6 | es No. 3 |
| Grade 40 | Reduction factor for seismic shear | <i>C</i> ℓv,seis | - | 0.70 | 0.70 | 0.80 | 0.80 | | | | |
| | Strength reduction factor for tension ² | ϕ | - | | | | 0. | 75 | | | |
| | Strength reduction factor for shear ² | ϕ | - | | | | 0. | 65 | | | |

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf.

1. Values provided for reinforcing bar material types based on minimum specified strengths and calculated in accordance with ACI 318-11 Eq. (D-2) and Eq. (D-29).

2. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC or ACI 318 Section 9.2 are used in accordance with ACI 318 D.4.3. If the load combinations of ACI 318 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to ductile steel elements. In accordance with ACI 318 D.3.3.4.3(a)6, deformed reinforcing bar meeting this specification used as ductile steel elements to resist earthquake effects shall be limited to reinforcing bars satisfying the requirements of ACI 318 section 21.1.5.2(a) and (b).

3. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC or ACI 318 Section 9.2 are used in accordance with ACI 318 D.4.3. If the load combinations of ACI 318 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4. Values correspond to brittle steel elements.

Concrete Breakout Design Information for Threaded Rod and Reinforcing Bars (For use with loads combinations taken from ACI 318 Section 9.2)¹



| | | | | | Nominal Rod | Diameter (i | nch) / Reinfor | cing Bar Size | 2 | |
|---|---------------------|--------------|---|---------------|------------------------------|---|--|----------------|-----------------|-----------------|
| Design Information | Symbol | Units | 3/8 or #3 | 1/2 or #4 | 5/8 or #5 | 3/4 or #6 | 7/8 or #7 | 1 or #8 | #9 | 1-1/4 or #10 |
| Effectiveness factor for cracked concrete | k _{c,cr} | - (SI) | Not Applicable | | | | 17 (7.1) | | | |
| Effectiveness factor for uncracked concrete | k _{c,uncr} | - (SI) | | | | | .4).0) | | | |
| Minimum embedment | h _{ef,min} | inch (mm) | 2-3/8 (60) | 2-3/4 (70) | 3-1/8 (79) | 3-1/2 (89) | 3-1/2 (89) | 4 (102) | 4-1/2 (114) | 5 (127) |
| Maximum embedment | h _{ef,max} | inch (mm) | 7-1/2 (191) | 10 (254) | 12-1/2 (318) | 15 (381) | 17-1/2 (445) | 20 (508) | 22-1/2 (572) | 25 (635) |
| Minimum anchor spacing | S _{min} | inch (mm) | 1-7/8 (48) | 2-1/2 (64) | 3-1/8 (79) | 3-3/4 (95) | 4-3/8 (111) | 5 (127) | 5-5/8 (143) | 6-1/4 (159) |
| Minimum edge distance ² | Cmin | inch (mm) | | | 5 <i>d</i> where <i>d</i> is | s nominal out | side diameter | of the anchor | | • |
| Minimum edge distance, reduced ² | Cmin, red | inch (mm) | 1-3/4 (45) | 1-3/4 (45) | 1-3/4 (45) | 1-3/4 (45) | 1-3/4 (45) | 1-3/4 (45) | 2-3/4 (70) | 2-3/4 (70) |
| Minimum member thickness | h _{min} | inch (mm) | h _{ef} + (h _{ef} + | | | h _{ef} ⊣ | ⊦ 2d₀ where d | ₀ is hole diam | eter; | |
| Critical edge distance—splitting (for | | inch | | | Cac | $= h_{ef} \cdot (\frac{\tau_{uncr}}{1160})$ | ^{₀.₄} · [3.1-0.7 <mark> </mark> | <u>1</u>] | | |
| uncracked concrete only) ³ | Cac | (mm) | | | Cac | $= h_{ef} \cdot (\frac{\tau_{uncr}}{8})$ | ^{₀.₄} · [3.1-0.7 | <u>]</u>] | | |
| Strength reduction factor for tension, concrete failure modes, Condition B ⁴ | φ | - | | | | 0. | 65 | | | |
| Strength reduction factor for shear, concrete failure modes, Condition B ⁴ | φ | - | | | | 0. | 70 | | | |

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N. For pound-inch units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf.

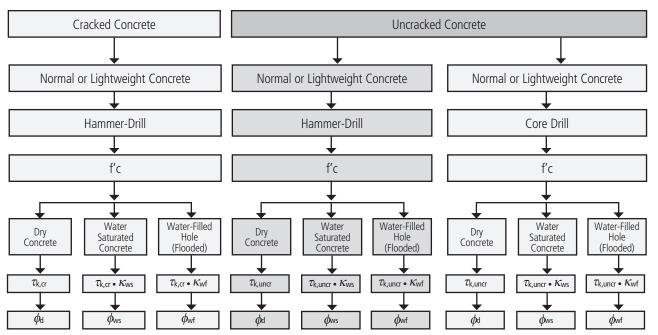
1. Additional setting information is described in the installation instructions.

2. For installation between the minimum edge distance, cmin, and the reduced minimum edge distance, cminved, the maximum torque applied must be reduced (multiplied) by a factor of 0.45.

3. $\tau_{k,uncr}$ need not be taken as greater than: $\tau_{k,uncr} \cdot \sqrt{h_{ef} \cdot f'_C} = \frac{h_{ef}}{\pi \cdot d}$ and $\frac{h}{h_{ef}}$ need not be taken as larger than 2.4.

4. Condition A requires supplemental reinforcement, while Condition B applies where supplemental reinforcement is not provided or where pryout governs, as set forth in ACI 318 D.4.3. The tabulated value of ϕ applies when the load combinations of Section 1605.2 of the IBC or ACI 318 Section 9.2 are used in accordance with ACI 318 D.4.4. If the load combinations of ACI 318 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318 D.4.4.

FLOWCHART FOR THE ESTABLISHMENT OF DESIGN BOND STRENGTH



FASTENING INNOVATIONS

Bond Strength Design Information for Threaded Rods and Reinforcing Bars in Holes Drilled with a Hammer Drill and Carbide Bit (For use with load combinations taken from ACI 318 Section 9.2)¹



| | | | | | Nom | inal Rod D | iameter (ir | nch) / Rein | forcing Bar | Size | |
|---|---|---------------------|----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|-----------------|---------------|
| Design In | formation | Symbol | Units | 3/8 or #3 | 1/2 or #4 | 5/8 or #5 | 3/4 or #6 | 7/8 or #7 | 1 or #8 | #9 | 11/4 o #10 |
| Minimum | embedment | h _{ef,min} | inch (mm) | 2-3/8 (60) | 2-3/4 (70) | 3-1/8 (79) | 3-1/2 (89) | 3-1/2 (89) | 4 (102 | 4-1/2 (114) | 5 (127) |
| Maximum embedment | Dry concrete and saturated concrete ⁷ | h _{ef,max} | inch (mm) | 4-1/2 (114) | 10 (254) | 12-1/2 (318) | 15 (381) | 17-1/2 (445) | 20 (508) | 22-1/2 (572) | 25 (635) |
| | Water-filled hole (flooded) | h _{ef,max} | inch (mm) | 4-1/2 (114) | 6 (152) | 7-1/2 (190) | 9 (225) | 10-1/2 (267) | 12 (305) | 13-1/2 (343) | 15 (381 |
| 75°F (24°C) Characteristic bond Maximum Long-Term strength in Service Temperature; cracked concrete ^{\$,8} | | $	au_{k,cr}$ | psi (N/mm²) | N/A | 1,119 (7.7) | 920 (6.3) | 857 (5 .9) | 807 (5.6) | 807 (5.6) | 807 (5.6) | 807 (5.6) |
| 104°F (40°C) Maximum Short-Term Service Temperature ^{4,10} | Characteristic bond strength in uncracked concrete ^{5,9} | $	au_{k,uncr}$ | psi (N/mm²) | 2,375 (16.4) | 2,244 (15.5) | 2,148 (14.8) | 2,073 (14.3) | 2,013 (13.9) | 1,960 (13.5) | 1,916 (13.2) | 1,87 (12.9 |
| 110°F (43°C) Maximum Long-Term Service Temperature; | Characteristic bond strength in cracked concrete ^{5,8} | $	au_{k,cr}$ | psi (N/mm²) | N/A | 576 (4.0) | 474 (3.3) | 441 (3.0) | 416 (2.9) | 416 (2.9) | 416 (2.9) | 416 (2.9 |
| 140°F (60°C) Maximum Short-Term Service Temperature ^{2,4} | Characteristic bond strength in uncracked concrete ^{5,9} | $	au_{k,uncr}$ | psi (N/mm²) | 1,223 (8.4) | 1,156 (8.0) | 1,106 (7.6) | 1,067 (7.4) | 1,036 (7.1) | 1,010 (7.0) | 986 (6.8) | 966 (6.7 |
| 110°F (43°C) Maximum Long-Term Service Temperature; | Characteristic bond strength in cracked concrete ^{5,8} | $	au_{k,cr}$ | psi (N/mm²) | N/A | 455 (3.1) | 374 (2.6) | 349 (2.4) | 329 (2.3) | 329 (2.3) | 329 (2.3) | 329 (2.3 |
| 176°F (80°C) Maximum Short-Term Service Temperature ^{3,4} | Characteristic bond strength in uncracked concrete ^{5,9} | $	au_{k,uncr}$ | psi (N/mm²) | 966 (6.7) | 913 (6.3) | 874 (6.0) | 843 (5.8) | 819 (5.6) | 798 (5.5) | 779 (5.4) | 763 (5.3 |
| | Dry concrete | $\phi_{ m d}$ | - | 0.65 | 0.65 | 0.65 | 0.65 | 0.65 | 0.65 | 0.65 | 0.65 |
| | Mater seturated sec- | $\phi_{ m ws}$ | - | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 |
| Permissible installation conditions ⁶ | Water-saturated concrete | Kws | | 0.93 | 0.9 | 0.96 | 1.0 | 1.0 | 1.0 | 1.0 | 0.99 |
| conditions | Water-filled hole (flooded) | $\phi_{ m wf}$ | - | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 |
| | Water-filled hole (flooded) | | | 0.93 | 0.83 | 0.75 | 0.70 | 0.65 | 0.62 | 0.59 | 0.56 |
| Reduction factor | $lpha_{\sf N,seis}$ | - | | | | 1. | .0 | | | | |

For SI: 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For pound-inch units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

1. Bond strength values correspond to a normal-weight concrete compressive strength f'c = 2,500 psi (17.2 MPa). For concrete compressive strength, f'c between 2,500 psi and 8,000 psi (17.2 MPa) and 55.2 MPa), the tabulated characteristic bond strength may be increased by a factor of (f'c / 2,500)^{0.12} [For SI: (f'c / 17.2)^{0.12}].

The maximum short-term service temperature may be increased to 162°F (72°C) provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.

3. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.

4. Short-term base material service temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term base material service temperatures are roughly constant over significant periods of time.

5. Characteristic bond strengths are for sustained loads including dead and live loads.

6. Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation.

7. Maximum embedment is limited to twelve anchor diameters for horizontal and upwardly inclined installations.

8. For structures assigned to Seismic Design Categories C, D, E or F, bond strength values for cracked concrete do not require an additional reduction factor applied for seismic tension ($\alpha_{N,seis} = 1.0$), where seismic design is applicable.

9. Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

10. Room temperature range is not recognized by ACI 318-11 and does not meet the minimum temperature requirement of ACI 355.4, Table 8.1 and consequently is not applicable to design under ACI 318-11 or current and past editions of the International Building Code (IBC). The tabulated values are provided for analysis and evaluation of existing conditions only.

Bond Strength Design Information for Threaded Rods and Reinforcing Bars in Holes Drilled with a Core Drill and Diamond Core Bit (For use with load combinations taken from ACI 318 Section 9.2)¹

| A BLES |
|--------|
| ABLES |

| Docian C | haracteristic | Notation | Units | N | ominal Rod Dia | meter (inch) / R | eforcing Bar Siz | e |
|--|--|-----------------------------------|----------------|----------------|-----------------|------------------|------------------|----------------|
| Design C | lidracteristic | NOTATION | Units | 1/2" or #4 | 5/8" or #5 | 3/4" or #6 | 7/8" or #7 | 1" or #8 |
| Minimum | embedment | hef,min | in. (mm) | 2-3/4 (70) | 3-1/8 (79) | 3-1/2 (89) | 3-1/2 (89) | 4 (102) |
| Maximum | embedment ⁷ | h _{ef,max} | in. (mm) | 10 (54) | 12-1/2 (318) | 15 (381) | 17-1/2 (445) | 20 (508) |
| 75°F (24°C) Maximum Long-Term Service Temperature; 104°F (40°C) Maximum Short-Term Service Temperature ⁴¹⁰ | aximum Long-Term ervice Temperature; 104°F (40°C) aximum Short-Term | | psi (N/mm²) | 1,419 (9.8) | 1,351 (9.3) | 1,298 (9.0) | 1,257 (8.7) | 1,221 (8.4) |
| 110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature ²⁴ | Characteristic bond strength in uncracked concrete ^{sa} | $\mathcal{T}_{k, uncr}$ | psi (N/mm²) | 1,133 (7.8) | 1,075 (7.4) | 1,033 (7.1) | 1,022 (6.9) | 975 (6.7) |
| 110°F (43°C) Maximum Long-Term Service Temperature; 176°F (80°C) Maximum Short-Term Service Temperature ³⁴ | Characteristic bond strength in uncracked concrete ^{s#} | $\mathcal{T}_{k,uncr}$ | psi (N/mm²) | 895 (6.2) | 849 (5.9) | 816 (5.6) | 791 (5.5) | 770 (5.3) |
| | Dry concrete | $\phi_{ m d}$ | - | 0.55 | 0.45 | 0.45 | 0.45 | 0.45 |
| | Water-saturated concrete | $\phi_{\scriptscriptstyle m ws}$ | - | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 |
| Permissible Installation Conditions ⁶ | vvater-saturated concrete | Kws | - | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |
| conditions | Water-filled hole (flooded) | $\phi_{ m ws}$ | - | 0.45 | 0.45 | 0.45 | 0.45 | 0.45 |
| | water-filled fible (fibbded) | $\kappa_{ m wf}$ | - | 0.94 | 0.95 | 0.95 | 0.95 | 0.96 |

For SI: 1 inch = 25.4 mm, 1 psi = 0.006894 MPa. For pound-inch units: 1 mm = 0.03937 inch, 1 MPa = 145.0 psi.

1. Bond strength values correspond to a normal-weight concrete compressive strength f'c = 2,500 psi (17.2 MPa). For concrete compressive strength, f'c between 2,500 psi and 8,000 psi (17.2 MPa and 55.2 MPa), the tabulated characteristic bond strength may be increased by a factor of (f'c / 2,500)^{0.12} [For SI: (f'c / 17.2)^{0.12}].

The maximum short-term service temperature may be increased to 162°F (72°C) provided characteristic bond strengths are reduced by 10 percent. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category B.

3. Long-term and short-term temperatures meet the requirements of Section 8.5 of ACI 355.4 and Table 8.1, Temperature Category A.

4. Short-term base material service temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling. Long-term base material service temperatures are roughly constant over significant periods of time.

5. Characteristic bond strengths are for sustained loads including dead and live loads.

EASTENING INNOVATION

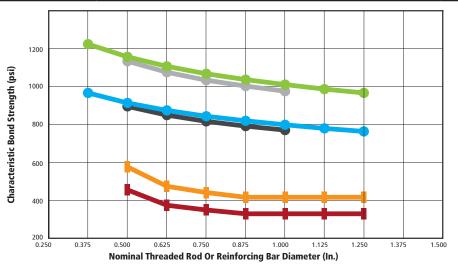
6. Permissible installation conditions include dry concrete, water-saturated concrete and water-filled holes. Water-filled holes include applications in dry or water-saturated concrete where the drilled holes contain standing water at the time of anchor installation.

7. Maximum embedment is limited to twelve anchor diameters for horizontal and upwardly inclined installations.

8. For structures assigned to Seismic Design Categories C, D, E or F, bond strength values for cracked concrete do not require an additional reduction factor applied for seismic tension (*α*_{N,seis} = 1.0), where seismic design is applicable.

9. Bond strength values for uncracked concrete are applicable for structures assigned to Seismic Design Categories A and B only.

10. Room temperature range is not recognized by ACI 318-11 and does not meet the minimum temperature requirement of ACI 355.4, Table 8.1 and consequently is not applicable to design under ACI 318-11 or current and past editions of the International Building Code (IBC). The tabulated values are provided for analysis and evaluation of existing conditions only.



Uncracked Concrete

 110°F Max Long-Term Service Temp, 140°F Max Short-Term ServiceTemp

 110°F Max Long-Term Service Temp, 176°F Max Short-Term ServiceTemp

Core Drilled Hole • 110°F Max Long-Term Service Temp, 140°F Max Short-Term ServiceTemp

Core Drilled Hole

• 110°F Max Long-Term Service Temp, 176°F Max Short-Term ServiceTemp

Cracked Concrete

110°F Max Long-Term Service Temp, 176°F Max Short-Term ServiceTemp

^{110°}F Max Long-Term Service Temp, 140°F Max Short-Term ServiceTemp

FASTENING INNOVATIONS

Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 75°F (24°C) Maximum Long-Term Service Temperature; 104°F (40°C) Maximum Short-Term Service Temperature ^{1,2,3,4,5,6,7,8}



| | | | | | Minimu | m Concrete C | ompressive S | trength | | | |
|--|----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal Rod/Rebar Size (in. or #) | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| | Depth h∉ (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/8 | 2,855 | 2,570 | 3,125 | 2,920 | 3,610 | 3,575 | 4,425 | 4,745 | 4,965 | 5,350 |
| 3/8 or #3 | 3 | 4,055 | 4,010 | 4,440 | 4,555 | 5,125 | 5,570 | 6,060 | 7,295 | 6,275 | 8,540 |
| | 4-1/2 | 7,445 | 7,935 | 8,155 | 9,015 | 8,660 | 10,660 | 9,090 | 13,315 | 9,410 | 15,585 |
| | 2-3/4 | 3,555 | 3,305 | 3,895 | 3,755 | 4,500 | 4,590 | 5,510 | 6,095 | 6,365 | 7,455 |
| 1/2 or #4 | 4 | 6,240 | 6,700 | 6,835 | 7,610 | 7,895 | 9,310 | 9,665 | 12,365 | 10,535 | 14,780 |
| 1/2 01 #4 | 6 | 11,465 | 13,235 | 12,560 | 15,035 | 14,500 | 18,390 | 15,270 | 22,995 | 15,805 | 26,920 |
| | 10 | 22,910 | 30,315 | 23,420 | 33,500 | 24,240 | 39,220 | 25,450 | 48,975 | 26,345 | 56,740 |
| | 3-1/8 | 4,310 | 4,120 | 4,720 | 4,680 | 5,450 | 5,720 | 6,675 | 7,600 | 7,710 | 9,295 |
| 5/8 or #5 | 5 | 8,720 | 9,985 | 9,555 | 11,345 | 11,030 | 13,875 | 13,510 | 18,430 | 15,600 | 22,540 |
| J/0 UI #J | 7-1/2 | 16,020 | 19,725 | 17,550 | 22,410 | 20,265 | 27,410 | 22,840 | 35,210 | 23,640 | 41,225 |
| | 12-1/2 | 34,270 | 46,440 | 35,025 | 51,320 | 36,255 | 60,085 | 38,065 | 75,035 | 39,400 | 84,865 |
| | 3-1/2 | 5,105 | 5,015 | 5,595 | 5,700 | 6,460 | 6,970 | 7,910 | 9,255 | 9,135 | 11,320 |
| 2/1 or #6 | 6 | 11,465 | 13,595 | 12,560 | 15,445 | 14,500 | 18,895 | 17,760 | 25,095 | 20,505 | 30,695 |
| 3/4 or #6 | 9 | 21,060 | 26,855 | 23,070 | 30,510 | 26,640 | 37,320 | 31,740 | 49,025 | 32,855 | 57,395 |
| | 15 | 45,315 | 63,370 | 48,675 | 71,435 | 50,385 | 83,635 | 52,900 | 104,445 | 54,755 | 117,935 |
| | 3-1/2 | 5,105 | 4,930 | 5,595 | 5,605 | 6,460 | 6,855 | 7,910 | 9,100 | 9,135 | 11,130 |
| 7/8 or #7 | 7 | 14,445 | 16,605 | 15,825 | 18,865 | 18,275 | 23,075 | 22,380 | 30,650 | 25,840 | 37,485 |
| //0 01 #/ | 10-1/2 | 26,540 | 32,800 | 29,070 | 37,265 | 33,570 | 45,580 | 41,115 | 60,540 | 43,425 | 71,450 |
| | 17-1/2 | 57,100 | 77,405 | 62,550 | 87,940 | 66,595 | 104,125 | 69,915 | 130,030 | 72,370 | 152,235 |
| | 4 | 6,240 | 6,115 | 6,835 | 6,945 | 7,895 | 8,495 | 9,665 | 11,280 | 11,160 | 13,800 |
| 1 or #0 | 8 | 17,650 | 19,750 | 19,335 | 22,435 | 22,325 | 27,440 | 27,340 | 36,450 | 31,570 | 44,580 |
| 1 or #8 | 12 | 32,425 | 39,005 | 35,520 | 44,315 | 41,015 | 54,200 | 50,230 | 71,990 | 55,225 | 86,340 |
| | 20 | 69,765 | 92,055 | 76,425 | 104,585 | 84,690 | 125,830 | 88,915 | 157,140 | 92,040 | 183,970 |
| | 4-1/2 | 7,445 | 7,110 | 8,155 | 8,080 | 9,420 | 9,880 | 11,535 | 13,125 | 13,320 | 16,055 |
| що | 9 | 21,060 | 23,055 | 23,070 | 26,190 | 26,640 | 32,035 | 32,625 | 42,550 | 37,675 | 52,040 |
| #9 | 13-1/2 | 38,690 | 45,540 | 42,380 | 51,740 | 48,940 | 63,280 | 59,940 | 84,050 | 68,320 | 102,275 |
| | 22-1/2 | 83,245 | 107,440 | 91,190 | 122,065 | 104,780 | 149,000 | 110,005 | 186,075 | 113,870 | 217,845 |
| | 5 | 8,720 | 8,170 | 9,555 | 9,285 | 11,030 | 11,355 | 13,510 | 15,085 | 15,600 | 18,450 |
| 1-1/4 | 10 | 24,665 | 26,380 | 27,020 | 29,975 | 31,200 | 36,660 | 38,210 | 48,690 | 44,125 | 59,555 |
| 1-1/4 | 15 | 45,315 | 52,110 | 49,640 | 59,200 | 57,320 | 72,410 | 70,200 | 96,175 | 81,060 | 117,630 |
| | 25 | 97,500 | 122,990 | 106,805 | 139,730 | 123,330 | 170,905 | 132,975 | 215,715 | 137,645 | 252,550 |
| | 5 | 8,720 | 8,160 | 9,555 | 9,270 | 11,030 | 11,335 | 13,510 | 15,060 | 15,600 | 18,420 |
| <i>щ</i> 10 | 10 | 24,665 | 26,430 | 27,020 | 30,025 | 31,200 | 36,725 | 38,210 | 48,780 | 44,125 | 59,660 |
| #10 | 15 | 45,315 | 52,205 | 49,640 | 59,310 | 57,320 | 72,545 | 70,200 | 96,350 | 81,060 | 117,845 |
| | 25 | 97,500 | 123,170 | 106,805 | 139,935 | 123,330 | 171,155 | 132,975 | 216,030 | 137,645 | 252,920 |

Concrete Breakout Strength - Bond Strength/Pryout Strength

 Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness, by by by and with the following conditions:

 $h_a = h_{min}$, and with the following conditions: - c_{a1} is greater than or equal to the critical edge distance, c_{ac}

- C_{a2} is greater than or equal to 1.5 times C_{a1} .

2. Calculations were performed following methodology in ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls. This temperature range is not recognized by ACI 318-11 and does not meet the minimum temperature requirements from ACI 355.4 Table 8.1 and consequently is not applicable to design under ACI 318-11 or current and past editions of the international building code (IBC). The tabulated values are provided for analysis and evaluation of existing conditions only.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for short-term static loads only, seismic loading is not considered with these tables.

6. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.
 I ong term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of

8. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.



Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature;

140°F (60°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}

| | | | | | Minimu | m Concrete C | Compressive S | trength | | | |
|---------------------------------|-----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | ,000 psi |
| Rod/Rebar Size (in. or #) | Depth hef (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/8 | 2,225 | 2,330 | 2,275 | 2,450 | 2,355 | 2,535 | 2,470 | 2,660 | 2,555 | 2,755 |
| 3/8 or #3 | 3 | 2,810 | 3,460 | 2,870 | 3,825 | 2,975 | 4,480 | 3,120 | 5,595 | 3,230 | 6,550 |
| | 4-1/2 | 4,215 | 6,320 | 4,310 | 6,985 | 4,460 | 8,175 | 4,680 | 10,085 | 4,845 | 10,435 |
| | 2-3/4 | 3,245 | 3,185 | 3,320 | 3,520 | 3,435 | 4,120 | 3,605 | 5,145 | 3,730 | 6,025 |
| 1/2 or #4 | 4 | 4,720 | 5,990 | 4,825 | 6,620 | 4,995 | 7,755 | 5,245 | 9,680 | 5,430 | 11,335 |
| 1/2 01 #4 | 6 | 7,080 | 10,915 | 7,240 | 12,065 | 7,495 | 14,125 | 7,865 | 16,945 | 8,145 | 17,540 |
| | 10 | 11,805 | 23,250 | 12,065 | 25,690 | 12,490 | 26,895 | 13,110 | 28,240 | 13,570 | 29,230 |
| | 3-1/8 | 4,310 | 4,120 | 4,510 | 4,595 | 4,665 | 5,375 | 4,900 | 6,715 | 5,070 | 7,860 |
| E/0 or #E | 5 | 7,060 | 9,175 | 7,215 | 10,140 | 7,465 | 11,870 | 7,840 | 14,825 | 8,115 | 17,355 |
| 5/8 or #5 | 7-1/2 | 10,585 | 16,710 | 10,820 | 18,465 | 11,200 | 21,620 | 11,760 | 25,330 | 12,170 | 26,220 |
| | 12-1/2 | 17,645 | 35,610 | 18,035 | 38,845 | 18,670 | 40,210 | 19,600 | 42,215 | 20,285 | 43,695 |
| | 3-1/2 | 5,105 | 5,015 | 5,480 | 5,700 | 5,735 | 6,790 | 6,000 | 8,480 | 6,195 | 9,925 |
| 2/4 or #6 | 6 | 9,805 | 12,775 | 10,020 | 14,115 | 10,375 | 16,525 | 10,890 | 20,635 | 11,275 | 24,160 |
| 3/4 or #6 | 9 | 14,705 | 23,265 | 15,035 | 25,710 | 15,560 | 30,100 | 16,335 | 35,185 | 16,910 | 36,420 |
| | 15 | 24,510 | 49,560 | 25,055 | 53,965 | 25,935 | 55,860 | 27,225 | 58,645 | 28,185 | 60,705 |
| | 3-1/2 | 5,085 | 4,930 | 5,290 | 5,605 | 5,625 | 6,855 | 5,980 | 8,765 | 6,175 | 10,260 |
| 7/8 or #7 | 7 | 12,960 | 15,900 | 13,245 | 17,570 | 13,710 | 20,570 | 14,395 | 25,690 | 14,900 | 30,075 |
| //0 01 #/ | 10-1/2 | 19,435 | 28,960 | 19,865 | 32,000 | 20,565 | 37,465 | 21,590 | 46,500 | 22,350 | 48,135 |
| | 17-1/2 | 32,395 | 61,700 | 33,110 | 68,185 | 34,275 | 73,820 | 35,985 | 77,500 | 37,245 | 80,225 |
| | 4 | 6,240 | 6,115 | 6,685 | 6,945 | 7,110 | 8,495 | 7,645 | 11,045 | 7,895 | 12,930 |
| 1 or #0 | 8 | 16,500 | 19,225 | 16,865 | 21,245 | 17,455 | 24,870 | 18,325 | 31,060 | 18,970 | 36,360 |
| 1 or #8 | 12 | 24,750 | 35,010 | 25,295 | 38,690 | 26,185 | 45,295 | 27,490 | 56,570 | 28,455 | 61,290 |
| | 20 | 41,250 | 74,605 | 42,160 | 82,440 | 43,640 | 94,000 | 45,820 | 98,685 | 47,430 | 102,150 |
| | 4-1/2 | 7,445 | 7,110 | 8,105 | 8,080 | 8,615 | 9,880 | 9,350 | 13,025 | 9,655 | 15,250 |
| #9 | 9 | 20,385 | 22,755 | 20,835 | 25,145 | 21,570 | 29,440 | 22,645 | 36,765 | 23,440 | 43,045 |
| #9 | 13-1/2 | 30,580 | 41,450 | 31,255 | 45,805 | 32,355 | 53,630 | 33,965 | 66,970 | 35,160 | 75,730 |
| | 22-1/2 | 50,965 | 88,290 | 52,095 | 97,570 | 53,920 | 114,230 | 56,610 | 121,930 | 58,600 | 126,215 |
| | 5 | 8,720 | 8,170 | 9,555 | 9,285 | 10,495 | 11,355 | 11,450 | 15,085 | 11,870 | 17,755 |
| 1-1/4 | 10 | 24,660 | 26,380 | 25,205 | 29,150 | 26,090 | 34,130 | 27,390 | 42,620 | 28,350 | 49,895 |
| 1-1/4 | 15 | 36,985 | 48,045 | 37,805 | 53,090 | 39,130 | 62,155 | 41,085 | 77,625 | 42,525 | 90,880 |
| | 25 | 61,645 | 102,380 | 63,005 | 113,140 | 65,220 | 132,460 | 68,470 | 147,480 | 70,875 | 152,660 |
| | 5 | 8,720 | 8,160 | 9,555 | 9,270 | 10,375 | 11,335 | 11,315 | 15,060 | 11,725 | 17,725 |
| #10 | 10 | 24,660 | 26,425 | 25,205 | 29,200 | 26,090 | 34,190 | 27,390 | 42,695 | 28,350 | 49,985 |
| #10 | 15 | 36,985 | 48,130 | 37,805 | 53,190 | 39,130 | 62,270 | 41,085 | 77,765 | 42,525 | 91,045 |
| | 25 | 61,645 | 102,530 | 63,005 | 113,305 | 65,220 | 132,655 | 68,470 | 147,480 | 70,875 | 152,660 |

🔲 - Concrete Breakout Strength 📃 - Bond Strength/Pryout Strength

Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness, 1. $h_a = h_{min}$, and with the following conditions: - c_{a1} is greater than or equal to the critical edge distance, c_{ac}

2. Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (\$\phi\$) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583. 8

Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of 9 diurnal cycling.

⁻ Ca2 is greater than or equal to 1.5 times Ca1.



Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 176°F (80°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}



| | | | | | Minimu | m Concrete C | Compressive S | trength | | | | |
|---------------------------------|-----------------------|---|--|---|--|---|--|---|--|---|--|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi | |
| Rod/Rebar Size (in. or #) | Depth hef (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | |
| | 2-3/8 | 1,755 | 1,890 | 1,795 | 1,935 | 1,860 | 2,000 | 1,950 | 2,100 | 2,020 | 2,175 | |
| 3/8 or #3 | 3 | 2,220 | 3,150 | 2,270 | 3,480 | 2,350 | 4,075 | 2,465 | 5,090 | 2,550 | 5,495 | |
| | 4-1/2 | 3,330 | 5,750 | 3,400 | 6,355 | 3,520 | 7,440 | 3,700 | 7,965 | 3,825 | 8,245 | |
| | 2-3/4 | 2,565 | 2,895 | 2,620 | 3,200 | 2,710 | 3,750 | 2,845 | 4,680 | 2,950 | 5,480 | |
| 1/2 or #4 | 4 | 3,730 | 5,455 | 3,810 | 6,025 | 3,945 | 7,055 | 4,140 | 8,810 | 4,285 | 9,235 | |
| 1/2 01 #4 | 6 | 5,595 | 9,935 | 5,715 | 10,975 | 5,920 | 12,745 | 6,215 | 13,380 | 6,430 | 13,850 | |
| | 10 | 9,320 | 20,080 | 9,530 | 20,520 | 9,865 | 21,245 | 10,355 | 22,300 | 10,720 | 23,085 | |
| | 3-1/8 | 3,485 | 3,785 | 3,565 | 4,180 | 3,690 | 4,895 | 3,870 | 6,110 | 4,010 | 7,155 | |
| 5/8 or #5 | 5 | 5,575 | 8,350 | 5,700 | 9,230 | 5,900 | 10,805 | 6,195 | 13,345 | 6,415 | 13,810 | |
| C# 10 8/C | 7-1/2 | 8,365 | 15,210 | 8,550 | 16,810 | 8,850 | 19,065 | 9,295 | 20,015 | 9,620 | 20,720 | |
| | 12-1/2 | 13,945 | 30,030 | 14,250 | 30,695 | 14,750 | 31,775 | 15,490 | 33,360 | 16,030 | 34,530 | |
| | 3-1/2 | 4,380 | 4,775 | 4,470 | 5,275 | 4,615 | 6,180 | 4,825 | 7,715 | 4,985 | 9,035 | |
| 2/4 #6 | 6 | 7,745 | 11,625 | 7,920 | 12,845 | 8,195 | 15,040 | 8,605 | 18,535 | 8,905 | 19,185 | |
| 3/4 or #6 | 9 | 11,620 | 21,170 | 11,875 | 23,395 | 12,295 | 26,480 | 12,905 | 27,800 | 13,360 | 28,775 | |
| | 15 | 19,365 | 41,710 | 19,795 | 42,635 | 20,490 | 44,130 | 21,510 | 46,330 | 22,265 | 47,960 | |
| | 3-1/2 | 4,355 | 4,930 | 4,450 | 5,455 | 4,595 | 6,390 | 4,805 | 7,975 | 4,960 | 9,340 | |
| 7/0 //7 | 7 | 10,245 | 14,475 | 10,470 | 15,995 | 10,840 | 18,725 | 11,380 | 23,385 | 11,780 | 25,370 | |
| 7/8 or #7 | 10-1/2 | 15,365 | 26,360 | 15,705 | 29,130 | 16,255 | 34,105 | 17,065 | 36,760 | 17,665 | 38,050 | |
| | 17-1/2 | 25,610 | 55,160 | 26,175 | 56,380 | 27,095 | 58,360 | 28,445 | 61,270 | 29,445 | 63,420 | |
| | 4 | 5,500 | 6,115 | 5,685 | 6,875 | 5,870 | 8,045 | 6,140 | 10,050 | 6,340 | 11,765 | |
| 4 110 | 8 | 13,035 | 17,495 | 13,325 | 19,335 | 13,795 | 22,635 | 14,480 | 28,265 | 14,990 | 32,285 | |
| 1 or #8 | 12 | 19,555 | 31,865 | 19,985 | 35,210 | 20,690 | 41,225 | 21,720 | 46,785 | 22,485 | 48,425 | |
| | 20 | 32,590 | 67,895 | 33,310 | 71,750 | 34,480 | 74,270 | 36,200 | 77,970 | 37,475 | 80,710 | |
| | 4-1/2 | 6,665 | 7,110 | 6,930 | 8,080 | 7,175 | 9,495 | 7,510 | 11,855 | 7,755 | 13,880 | |
| | 9 | 16,105 | 20,710 | 16,465 | 22,885 | 17,040 | 26,795 | 17,890 | 33,460 | 18,520 | 39,170 | |
| #9 | 13-1/2 | 24,160 | 37,720 | 24,695 | 41,685 | 25,560 | 48,805 | 26,835 | 57,800 | 27,780 | 59,830 | |
| | 22-1/2 | 40,265 | 80,350 | 41,155 | 88,645 | 42,600 | 91,760 | 44,725 | 96,335 | 46,295 | 99,715 | |
| | 5 | 8,115 | 8,170 | 8,445 | 9,285 | 8,820 | 11,050 | 9,230 | 13,800 | 9,530 | 16,155 | |
| | 10 | 19,475 | 24,005 | 19,905 | 26,525 | 20,605 | 31,055 | 21,635 | 38,780 | 22,395 | 45,405 | |
| 1-1/4 | 15 | 29,215 | 43,715 | 29,860 | 48,310 | 30,910 | 56,560 | 32,450 | 69,890 | 33,590 | 72,345 | |
| | 25 | 48,690 | 93,160 | 49,765 | 102,950 | 51,515 | 110,955 | 54,085 | 116,485 | 55,985 | 120,580 | |
| | 5 | 8,020 | 8,160 | 8,345 | 9,270 | 8,715 | 11,030 | 9,120 | 13,775 | 9,415 | 16,130 | |
| | 10 | 19,475 | 24,045 | 19,905 | 26,570 | 20,605 | 31,110 | 21,635 | 38,850 | 22,395 | 45,485 | |
| #10 | 15 | 29,215 | 43,800 | 29,860 | 48,400 | 30,910 | 56,665 | 32,450 | 69,890 | 33,590 | 72,345 | |
| - | 25 | 48,690 | 93,300 | 49,765 | 103,100 | 51,515 | 110,955 | 54,085 | 116,485 | 55,985 | 120,580 | |

🔲 - Concrete Breakout Strength 📃 - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness,

 $h_a = h_{min}$, and with the following conditions: - c_{a1} is greater than or equal to the critical edge distance, c_{ac}

- C_{a2} is greater than or equal to 1.5 times C_{a1} .

 Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

 Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

8. Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.



Tension and Shear Design Strength Installed in Cracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 75°F (24°C) Maximum Long-Term Service Temperature; 104°F (40°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8}



| | | | | | Minimu | m Concrete C | ompressive S | trength | | | |
|---------------------------------|-----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth hef (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 2,520 | 2,360 | 2,760 | 2,680 | 3,030 | 3,280 | 3,320 | 4,355 | 3,545 | 5,325 |
| 1/2 or #4 | 4 | 4,420 | 4,785 | 4,670 | 5,435 | 4,835 | 6,650 | 5,075 | 8,830 | 5,255 | 10,555 |
| 1/2 01 #4 | 6 | 6,855 | 9,455 | 7,005 | 10,740 | 7,255 | 13,135 | 7,615 | 16,400 | 7,880 | 16,975 |
| | 10 | 11,425 | 21,650 | 11,680 | 23,930 | 12,090 | 26,035 | 12,690 | 27,335 | 13,135 | 28,295 |
| | 3-1/8 | 2,910 | 2,940 | 3,025 | 3,340 | 3,220 | 4,085 | 3,515 | 5,430 | 3,745 | 6,640 |
| E/0 or #E | 5 | 5,870 | 7,135 | 6,000 | 8,105 | 6,210 | 9,910 | 6,520 | 13,165 | 6,750 | 14,540 |
| 5/8 or #5 | 7-1/2 | 8,805 | 14,090 | 9,000 | 16,005 | 9,315 | 19,575 | 9,780 | 21,070 | 10,125 | 21,810 |
| | 12-1/2 | 14,675 | 31,610 | 15,000 | 32,310 | 15,530 | 33,445 | 16,305 | 35,115 | 16,875 | 36,345 |
| | 3-1/2 | 3,375 | 3,580 | 3,500 | 4,070 | 3,720 | 4,980 | 4,050 | 6,610 | 4,305 | 8,085 |
| 2/4 1/6 | 6 | 7,875 | 9,710 | 8,050 | 11,035 | 8,330 | 13,495 | 8,745 | 17,925 | 9,055 | 19,500 |
| 3/4 or #6 | 9 | 11,815 | 19,185 | 12,075 | 21,795 | 12,500 | 26,655 | 13,120 | 28,260 | 13,580 | 29,255 |
| | 15 | 19,690 | 42,405 | 20,125 | 43,340 | 20,830 | 44,865 | 21,870 | 47,100 | 22,635 | 48,755 |
| 7/8 or #7 - | 3-1/2 | 3,265 | 3,525 | 3,380 | 4,000 | 3,575 | 4,895 | 3,875 | 6,500 | 4,105 | 7,950 |
| | 7 | 10,095 | 11,860 | 10,315 | 13,475 | 10,680 | 16,485 | 11,210 | 21,895 | 11,605 | 24,995 |
| //8 OF #/ | 10-1/2 | 15,140 | 23,430 | 15,475 | 26,620 | 16,020 | 32,555 | 16,815 | 36,220 | 17,410 | 37,495 |
| | 17-1/2 | 25,235 | 54,350 | 25,790 | 55,550 | 26,700 | 57,505 | 28,030 | 60,370 | 29,015 | 62,490 |
| | 4 | 4,240 | 4,365 | 4,390 | 4,960 | 4,645 | 6,065 | 5,030 | 8,060 | 5,330 | 9,855 |
| 1 110 | 8 | 12,500 | 14,105 | 13,475 | 16,025 | 13,950 | 19,600 | 14,645 | 26,035 | 15,160 | 31,845 |
| 1 or #8 | 12 | 19,775 | 27,860 | 20,210 | 31,655 | 20,920 | 38,715 | 21,965 | 47,310 | 22,735 | 48,970 |
| | 20 | 32,960 | 65,755 | 33,685 | 72,560 | 34,870 | 75,105 | 36,610 | 78,850 | 37,895 | 81,620 |
| | 4-1/2 | 5,275 | 5,080 | 5,475 | 5,770 | 5,785 | 7,060 | 6,265 | 9,375 | 6,630 | 11,465 |
| | 9 | 14,920 | 16,465 | 16,340 | 18,710 | 17,655 | 22,880 | 18,535 | 30,390 | 19,185 | 37,170 |
| #9 | 13-1/2 | 25,030 | 32,530 | 25,580 | 36,955 | 26,480 | 45,200 | 27,800 | 59,875 | 28,775 | 61,980 |
| | 22-1/2 | 41,715 | 76,740 | 42,635 | 87,190 | 44,135 | 95,055 | 46,335 | 99,795 | 47,960 | 103,300 |
| | 5 | 6,175 | 5,835 | 6,765 | 6,630 | 7,190 | 8,110 | 7,785 | 10,775 | 8,240 | 13,175 |
| 1 1/4 | 10 | 17,470 | 18,845 | 19,140 | 21,410 | 21,795 | 26,185 | 22,880 | 34,780 | 23,685 | 42,540 |
| 1-1/4 | 15 | 30,900 | 37,220 | 31,580 | 42,285 | 32,690 | 51,720 | 34,320 | 68,695 | 35,525 | 76,520 |
| - | 25 | 51,500 | 87,850 | 52,635 | 99,810 | 54,485 | 117,355 | 57,200 | 123,205 | 59,210 | 127,530 |
| | 5 | 6,175 | 5,830 | 6,735 | 6,620 | 7,120 | 8,100 | 7,705 | 10,755 | 8,155 | 13,155 |
| #10 | 10 | 17,470 | 18,880 | 19,140 | 21,445 | 21,795 | 26,230 | 22,880 | 34,840 | 23,685 | 42,615 |
| #10 | 15 | 30,900 | 37,290 | 31,580 | 42,365 | 32,690 | 51,815 | 34,320 | 68,825 | 35,525 | 76,520 |
| | 25 | 51,500 | 87,980 | 52,635 | 99,955 | 54,485 | 117,355 | 57,200 | 123,205 | 59,210 | 127,530 |

🔲 - Concrete Breakout Strength 🔲 - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in cracked normal-weight concrete with minimum slab thickness,

 $h_a = h_{min}$, and with the following conditions:

- c_{a1} is greater than or equal to the critical edge distance, c_{ac}

- C_{a2} is greater than or equal to 1.5 times C_{a1} .

2. Calculations were performed following methodology in ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls. This temperature range is not recognized by ACI 318-11 and does not meet the minimum temperature requirements from ACI 355.4 Table 8.1 and consequently is not applicable to design under ACI 318-11 or current and past editions of the international building code (IBC). The tabulated values are provided for analysis and evaluation of existing conditions only.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for short-term static loads only, seismic loading is not considered with these tables.

6. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

 Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

8. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.

FASTENING INNOVATIONS

Tension and Shear Design Strength Installed in Cracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 140°F (60°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}

| | | | | | Minimu | imum Concrete Compressive Strength | | | | | |
|---------------------------------|----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | .000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth h∉ (in.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 1,615 | 2,275 | 1,655 | 2,515 | 1,710 | 2,945 | 1,795 | 3,675 | 1,860 | 4,005 |
| 1/2 or #4 | 4 | 2,350 | 4,280 | 2,405 | 4,730 | 2,490 | 5,360 | 2,615 | 5,630 | 2,705 | 5,825 |
| 1/2 01 #4 | 6 | 3,530 | 7,600 | 3,605 | 7,770 | 3,735 | 8,040 | 3,920 | 8,440 | 4,055 | 8,740 |
| | 10 | 5,880 | 12,665 | 6,010 | 12,945 | 6,220 | 13,400 | 6,535 | 14,070 | 6,760 | 14,565 |
| | 3-1/8 | 1,890 | 2,940 | 1,930 | 3,280 | 2,000 | 3,840 | 2,100 | 4,525 | 2,175 | 4,680 |
| 5/8 or #5 | 5 | 3,025 | 6,515 | 3,090 | 6,660 | 3,200 | 6,895 | 3,360 | 7,235 | 3,480 | 7,490 |
| 5/6 01 #5 | 7-1/2 | 4,535 | 9,770 | 4,640 | 9,990 | 4,800 | 10,340 | 5,040 | 10,855 | 5,215 | 11,235 |
| | 12-1/2 | 7,560 | 16,285 | 7,730 | 16,645 | 8,000 | 17,230 | 8,400 | 18,090 | 8,695 | 18,725 |
| | 3-1/2 | 2,175 | 3,580 | 2,265 | 4,070 | 2,370 | 4,850 | 2,480 | 5,340 | 2,560 | 5,515 |
| 3/4 or #6 | 6 | 4,050 | 8,730 | 4,140 | 8,920 | 4,290 | 9,235 | 4,500 | 9,695 | 4,660 | 10,035 |
| 5/4 01 #0 | 9 | 6,080 | 13,090 | 6,215 | 13,380 | 6,430 | 13,850 | 6,750 | 14,545 | 6,990 | 15,055 |
| | 15 | 10,130 | 21,820 | 10,355 | 22,305 | 10,720 | 23,085 | 11,255 | 24,240 | 11,650 | 25,090 |
| | 3-1/2 | 2,045 | 3,525 | 2,125 | 4,000 | 2,260 | 4,865 | 2,400 | 5,170 | 2,480 | 5,340 |
| 7/8 or #7 | 7 | 5,205 | 11,205 | 5,320 | 11,455 | 5,505 | 11,855 | 5,780 | 12,450 | 5,980 | 12,885 |
| //o UI #/ | 10-1/2 | 7,805 | 16,810 | 7,975 | 17,180 | 8,255 | 17,785 | 8,670 | 18,670 | 8,975 | 19,330 |
| | 17-1/2 | 13,010 | 28,015 | 13,295 | 28,635 | 13,760 | 29,640 | 14,450 | 31,120 | 14,955 | 32,215 |
| | 4 | 2,650 | 4,365 | 2,755 | 4,960 | 2,930 | 6,065 | 3,150 | 6,780 | 3,250 | 7,005 |
| 1 or #8 | 8 | 6,795 | 13,730 | 6,945 | 14,960 | 7,190 | 15,485 | 7,550 | 16,260 | 7,815 | 16,830 |
| I UI #0 | 12 | 10,195 | 21,955 | 10,420 | 22,440 | 10,785 | 23,230 | 11,325 | 24,390 | 11,720 | 25,245 |
| | 20 | 16,990 | 36,595 | 17,365 | 37,405 | 17,975 | 38,715 | 18,870 | 40,645 | 19,535 | 42,075 |
| | 4-1/2 | 3,290 | 5,080 | 3,420 | 5,770 | 3,635 | 7,060 | 3,945 | 8,495 | 4,075 | 8,775 |
| #9 | 9 | 8,600 | 16,255 | 8,790 | 17,960 | 9,100 | 19,600 | 9,555 | 20,575 | 9,890 | 21,300 |
| #9 | 13-1/2 | 12,900 | 27,790 | 13,185 | 28,405 | 13,650 | 29,400 | 14,330 | 30,865 | 14,835 | 31,950 |
| | 22-1/2 | 21,505 | 46,315 | 21,980 | 47,340 | 22,750 | 49,000 | 23,885 | 51,445 | 24,725 | 53,250 |
| | 5 | 4,090 | 5,835 | 4,250 | 6,630 | 4,520 | 8,110 | 4,930 | 10,620 | 5,110 | 11,010 |
| 1-1/4 | 10 | 10,620 | 18,840 | 10,855 | 20,820 | 11,235 | 24,200 | 11,795 | 25,405 | 12,210 | 26,295 |
| 1-1/4 | 15 | 15,930 | 34,305 | 16,280 | 35,065 | 16,850 | 36,295 | 17,690 | 38,105 | 18,315 | 39,445 |
| | 25 | 26,545 | 57,175 | 27,135 | 58,440 | 28,085 | 60,495 | 29,485 | 63,510 | 30,525 | 65,740 |
| | 5 | 4,045 | 5,830 | 4,205 | 6,620 | 4,465 | 8,100 | 4,870 | 10,495 | 5,050 | 10,880 |
| #10 | 10 | 10,620 | 18,875 | 10,855 | 20,860 | 11,235 | 24,200 | 11,795 | 25,405 | 12,210 | 26,295 |
| #10 | 15 | 15,930 | 34,305 | 16,280 | 35,065 | 16,850 | 36,295 | 17,690 | 38,105 | 18,315 | 39,445 |
| | 25 | 26,545 | 57,175 | 27,135 | 58,440 | 28,085 | 60,495 | 29,485 | 63,510 | 30,525 | 65,740 |

Concrete Breakout Strength - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in cracked normal-weight concrete with minimum slab thickness,

 $h_a = h_{min}$, and with the following conditions:

- c_{a1} is greater than or equal to the critical edge distance, c_{a2} - c_{a2} is greater than or equal to 1.5 times c_{a1} .

2. Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

 Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.



Tension and Shear Design Strength Installed in Cracked Concrete (Bond or Concrete Strength) Drilled with a Hammer-Drill and Carbide Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature;

176°F (80°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}

| | | | | | Minimu | m Concrete C | ompressive S | trength | | | |
|---------------------------------|----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth h∉ (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 1,280 | 2,070 | 1,305 | 2,285 | 1,350 | 2,680 | 1,420 | 3,055 | 1,470 | 3,165 |
| 1/2 #4 | 4 | 1,860 | 3,895 | 1,900 | 4,090 | 1,965 | 4,235 | 2,065 | 4,445 | 2,135 | 4,600 |
| 1/2 or #4 | 6 | 2,785 | 6,005 | 2,850 | 6,135 | 2,950 | 6,350 | 3,095 | 6,670 | 3,205 | 6,905 |
| | 10 | 4,645 | 10,005 | 4,750 | 10,225 | 4,915 | 10,585 | 5,160 | 11,115 | 5,340 | 11,505 |
| | 3-1/8 | 1,490 | 2,700 | 1,525 | 2,985 | 1,580 | 3,400 | 1,655 | 3,570 | 1,715 | 3,695 |
| E/0 or #E | 5 | 2,385 | 5,140 | 2,440 | 5,255 | 2,525 | 5,440 | 2,650 | 5,710 | 2,745 | 5,910 |
| 5/8 or #5 | 7-1/2 | 3,580 | 7,710 | 3,660 | 7,880 | 3,790 | 8,160 | 3,975 | 8,565 | 4,115 | 8,865 |
| | 12-1/2 | 5,965 | 12,850 | 6,100 | 13,135 | 6,315 | 13,595 | 6,630 | 14,275 | 6,860 | 14,775 |
| | 3-1/2 | 1,815 | 3,410 | 1,850 | 3,770 | 1,910 | 4,115 | 2,000 | 4,305 | 2,065 | 4,445 |
| 2/4 or #6 | 6 | 3,205 | 6,905 | 3,280 | 7,060 | 3,395 | 7,310 | 3,560 | 7,675 | 3,685 | 7,940 |
| 3/4 or #6 | 9 | 4,810 | 10,360 | 4,915 | 10,590 | 5,090 | 10,960 | 5,345 | 11,510 | 5,530 | 11,915 |
| | 15 | 8,020 | 17,270 | 8,195 | 17,650 | 8,485 | 18,270 | 8,905 | 19,180 | 9,220 | 19,855 |
| | 3-1/2 | 1,750 | 3,525 | 1,785 | 3,850 | 1,845 | 3,975 | 1,930 | 4,160 | 1,995 | 4,295 |
| 7/8 or #7 | 7 | 4,115 | 8,865 | 4,205 | 9,060 | 4,355 | 9,375 | 4,570 | 9,845 | 4,730 | 10,190 |
| //8 01 #/ | 10-1/2 | 6,170 | 13,295 | 6,310 | 13,590 | 6,530 | 14,065 | 6,855 | 14,765 | 7,095 | 15,285 |
| | 17-1/2 | 10,285 | 22,155 | 10,515 | 22,650 | 10,885 | 23,445 | 11,425 | 24,610 | 11,830 | 25,475 |
| | 4 | 2,270 | 4,365 | 2,345 | 4,910 | 2,420 | 5,210 | 2,530 | 5,450 | 2,615 | 5,630 |
| 1 or #8 | 8 | 5,375 | 11,575 | 5,495 | 11,830 | 5,685 | 12,250 | 5,970 | 12,860 | 6,180 | 13,310 |
| 1 01 #8 | 12 | 8,060 | 17,365 | 8,240 | 17,750 | 8,530 | 18,370 | 8,955 | 19,290 | 9,270 | 19,965 |
| | 20 | 13,435 | 28,940 | 13,735 | 29,580 | 14,215 | 30,620 | 14,925 | 32,145 | 15,450 | 33,275 |
| | 4-1/2 | 2,815 | 5,080 | 2,930 | 5,770 | 3,030 | 6,530 | 3,170 | 6,830 | 3,275 | 7,055 |
| #9 | 9 | 6,800 | 14,650 | 6,955 | 14,975 | 7,195 | 15,500 | 7,555 | 16,275 | 7,820 | 16,845 |
| #9 | 13-1/2 | 10,205 | 21,975 | 10,430 | 22,465 | 10,795 | 23,250 | 11,335 | 24,410 | 11,730 | 25,270 |
| | 22-1/2 | 17,005 | 36,630 | 17,380 | 37,440 | 17,990 | 38,755 | 18,890 | 40,685 | 19,555 | 42,115 |
| | 5 | 3,500 | 5,835 | 3,640 | 6,630 | 3,805 | 7,895 | 3,980 | 8,570 | 4,110 | 8,850 |
| 1-1/4 | 10 | 8,400 | 17,145 | 8,585 | 18,490 | 8,885 | 19,135 | 9,330 | 20,090 | 9,655 | 20,795 |
| 1-1/4 | 15 | 12,595 | 27,130 | 12,875 | 27,730 | 13,330 | 28,705 | 13,990 | 30,135 | 14,485 | 31,195 |
| | 25 | 20,995 | 45,220 | 21,460 | 46,220 | 22,215 | 47,845 | 23,320 | 50,230 | 24,140 | 51,995 |
| | 5 | 3,460 | 5,830 | 3,595 | 6,620 | 3,755 | 7,880 | 3,930 | 8,470 | 4,060 | 8,745 |
| #10 | 10 | 8,400 | 17,175 | 8,585 | 18,490 | 8,885 | 19,135 | 9,330 | 20,090 | 9,655 | 20,795 |
| # IU | 15 | 12,595 | 27,130 | 12,875 | 27,730 | 13,330 | 28,705 | 13,990 | 30,135 | 14,485 | 31,195 |
| | 25 | 20,995 | 45,220 | 21,460 | 46,220 | 22,215 | 47,845 | 23,320 | 50,230 | 24,140 | 51,995 |

🔲 - Concrete Breakout Strength 🛄 - Bond Strength/Pryout Strength

 Tabular values are provided for illustration and are applicable for single anchors installed in cracked normal-weight concrete with minimum slab thickness, h_a = h_{min}, and with the following conditions:

 $- c_{a1}$ is greater than or equal to the critical edge distance, c_{ac}

- Ca2 is greater than or equal to 1.5 times Ca1.

 Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

 Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.



Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Core-Drill and Diamond Core Bit in a Dry Hole Condition 75°F (24°C) Maximum Long-Term Service Temperature; 104°F (40°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8}

| | | | | | Minimu | m Concrete C | ompressive S | trength | | | |
|---------------------------------|-----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth hef (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 3,370 | 3,305 | 3,445 | 3,755 | 3,565 | 4,470 | 3,745 | 5,585 | 3,875 | 6,540 |
| 1/2 or #4 | 4 | 4,905 | 6,505 | 5,010 | 7,190 | 5,190 | 8,415 | 5,445 | 10,510 | 5,640 | 12,305 |
| 1/2 01 #4 | 6 | 7,355 | 11,850 | 7,520 | 13,095 | 7,780 | 15,330 | 8,170 | 19,145 | 8,455 | 21,530 |
| | 10 | 12,260 | 25,235 | 12,530 | 27,890 | 12,970 | 32,650 | 13,615 | 34,660 | 14,095 | 35,880 |
| | 3-1/8 | 3,450 | 4,120 | 3,600 | 4,680 | 3,840 | 5,725 | 4,075 | 7,275 | 4,205 | 8,520 |
| 5/8 or #5 | 5 | 5,970 | 9,960 | 6,100 | 11,005 | 6,315 | 12,885 | 6,630 | 16,090 | 6,865 | 18,835 |
| C# 10 8/C | 7-1/2 | 8,955 | 18,135 | 9,150 | 20,045 | 9,470 | 23,465 | 9,945 | 29,305 | 10,295 | 32,025 |
| | 12-1/2 | 14,920 | 38,635 | 15,250 | 42,695 | 15,785 | 49,115 | 16,575 | 51,565 | 17,155 | 53,375 |
| | 3-1/2 | 4,125 | 5,015 | 4,295 | 5,700 | 4,575 | 6,970 | 4,980 | 9,170 | 5,140 | 10,735 |
| 2/4 or #6 | 6 | 8,260 | 13,595 | 8,440 | 15,265 | 8,735 | 17,870 | 9,170 | 22,320 | 9,495 | 26,130 |
| 3/4 or #6 | 9 | 12,385 | 25,160 | 12,660 | 27,805 | 13,105 | 32,550 | 13,760 | 40,650 | 14,240 | 44,305 |
| | 15 | 20,645 | 53,605 | 21,100 | 59,235 | 21,840 | 67,950 | 22,930 | 71,340 | 23,735 | 73,845 |
| | 3-1/2 | 4,020 | 4,930 | 4,175 | 5,605 | 4,430 | 6,855 | 4,830 | 9,100 | 5,120 | 11,085 |
| 7/0 or #7 | 7 | 10,885 | 16,605 | 11,125 | 18,865 | 11,515 | 22,225 | 12,090 | 27,755 | 12,515 | 32,495 |
| 7/8 or #7 | 10-1/2 | 16,325 | 31,290 | 16,690 | 34,575 | 17,275 | 40,480 | 18,135 | 50,550 | 18,770 | 58,400 |
| | 17-1/2 | 27,210 | 66,665 | 27,815 | 73,670 | 28,790 | 86,245 | 30,225 | 94,035 | 31,285 | 97,335 |
| | 4 | 5,070 | 6,115 | 5,265 | 6,945 | 5,590 | 8,495 | 6,090 | 11,280 | 6,475 | 13,800 |
| 1 or #0 | 8 | 13,810 | 19,750 | 14,115 | 22,435 | 14,610 | 26,830 | 15,340 | 33,510 | 15,880 | 39,230 |
| 1 or #8 | 12 | 20,715 | 37,770 | 21,170 | 41,740 | 21,915 | 48,870 | 23,010 | 61,030 | 23,815 | 71,450 |
| | 20 | 34,525 | 80,485 | 35,285 | 88,940 | 36,525 | 104,130 | 38,345 | 119,300 | 39,695 | 123,495 |

🔲 - Concrete Breakout Strength 🔲 - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness,

 $\begin{array}{l} \mbox{radiation} \mbox{radiation$

Calculations were performed following methodology in ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls. This temperature range is not recognized by ACI 318-11 and does not meet the minimum temperature requirements from ACI 355.4 Table 8.1 and consequently is not 2. applicable to design under ACI 318-11 or current and past editions of the international building code (IBC). The tabulated values are provided for analysis and evaluation of existing conditions only.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (ϕ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for short-term static loads only, seismic loading is not considered with these tables.

6. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583. 7.

Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of 8. diurnal cycling



Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength)



Drilled with a Core-Drill and Diamond Core Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature;

140°F (60°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}

| | | | | | Minimu | m Concrete C | ompressive S | trength | | | |
|---------------------------------|----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth h∉ (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 2,690 | 3,160 | 2,750 | 3,490 | 2,850 | 4,085 | 2,990 | 5,105 | 3,095 | 5,975 |
| 1/2 #4 | 4 | 3,915 | 5,945 | 4,000 | 6,570 | 4,145 | 7,690 | 4,350 | 9,605 | 4,500 | 11,245 |
| 1/2 or #4 | 6 | 5,875 | 10,830 | 6,005 | 11,965 | 6,215 | 14,010 | 6,525 | 16,605 | 6,755 | 17,190 |
| | 10 | 9,790 | 23,065 | 10,005 | 25,465 | 10,355 | 26,360 | 10,875 | 27,675 | 11,255 | 28,650 |
| | 3-1/8 | 2,970 | 4,110 | 3,035 | 4,540 | 3,140 | 5,320 | 3,295 | 6,640 | 3,410 | 7,775 |
| 5/8 or #5 | 5 | 4,750 | 9,090 | 4,855 | 10,045 | 5,025 | 11,760 | 5,275 | 14,685 | 5,460 | 16,990 |
| 5/6 01 #5 | 7-1/2 | 7,125 | 16,555 | 7,280 | 18,290 | 7,535 | 21,415 | 7,915 | 24,620 | 8,190 | 25,485 |
| | 12-1/2 | 11,875 | 35,260 | 12,135 | 37,755 | 12,560 | 39,080 | 13,190 | 41,030 | 13,650 | 42,470 |
| | 3-1/2 | 3,570 | 5,015 | 3,720 | 5,700 | 3,855 | 6,700 | 4,030 | 8,370 | 4,160 | 9,800 |
| 3/4 or #6 | 6 | 6,570 | 12,610 | 6,715 | 13,935 | 6,955 | 16,310 | 7,300 | 20,370 | 7,555 | 23,510 |
| 5/4 01 #0 | 9 | 9,855 | 22,965 | 10,075 | 25,375 | 10,430 | 29,710 | 10,950 | 34,065 | 11,335 | 35,260 |
| | 15 | 16,430 | 48,925 | 16,795 | 52,245 | 17,380 | 54,080 | 18,250 | 56,775 | 18,890 | 58,770 |
| | 3-1/2 | 3,445 | 4,930 | 3,580 | 5,605 | 3,810 | 6,855 | 4,015 | 8,645 | 4,145 | 10,125 |
| 7/8 or #7 | 7 | 8,675 | 15,690 | 8,870 | 17,340 | 9,180 | 20,300 | 9,635 | 25,350 | 9,975 | 29,675 |
| //o UI #/ | 10-1/2 | 13,015 | 28,575 | 13,300 | 31,580 | 13,770 | 36,970 | 14,455 | 44,975 | 14,965 | 46,555 |
| | 17-1/2 | 21,690 | 60,885 | 22,170 | 67,280 | 22,950 | 71,400 | 24,095 | 74,960 | 24,940 | 77,590 |
| | 4 | 4,350 | 6,115 | 4,520 | 6,945 | 4,810 | 8,495 | 5,120 | 10,890 | 5,290 | 12,745 |
| 1 or #0 | 8 | 11,025 | 18,955 | 11,270 | 20,945 | 11,665 | 24,520 | 12,250 | 30,625 | 12,680 | 35,855 |
| 1 or #8 | 12 | 16,540 | 34,520 | 16,905 | 38,150 | 17,500 | 44,665 | 18,375 | 55,775 | 19,020 | 59,165 |
| | 20 | 27,565 | 73,560 | 28,175 | 81,285 | 29,165 | 90,740 | 30,620 | 95,265 | 31,695 | 98,610 |

🔲 - Concrete Breakout Strength 📃 - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:

- Ca1 is greater than or equal to the critical edge distance, Cac

- Ca2 is greater than or equal to 1.5 times Ca1.

2. Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (ϕ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

 Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.



Tension and Shear Design Strength Installed in Uncracked Concrete (Bond or Concrete Strength) Drilled with a Core-Drill and Diamond Core Bit in a Dry Hole Condition 110°F (43°C) Maximum Long-Term Service Temperature; 176°F (80°C) Maximum Short-Term Service Temperature^{1,2,3,4,5,6,7,8,9}

| | | | | | Minimu | Im Concrete Compressive Strength | | | | | |
|---------------------------------|----------------------|---|--|---|--|---|--|---|--|---|--|
| Nominal Bod/Bobar | Embed. | f'c = 2, | 500 psi | f'c = 3, | 000 psi | f'c = 4, | 000 psi | f'c = 6, | 000 psi | f'c = 8, | 000 psi |
| Rod/Rebar Size (in. or #) | Depth h∉ (in.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | Φ Ncb or Φ Na Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) | $\Phi_{ m Ncb}$ or $\Phi_{ m Na}$ Tension (lbs.) | Φ Vcb or Φ Vcp Shear (lbs.) |
| | 2-3/4 | 2,125 | 2,875 | 2,175 | 3,175 | 2,250 | 3,720 | 2,360 | 4,645 | 2,445 | 5,435 |
| 1/2 or #4 | 4 | 3,095 | 5,410 | 3,160 | 5,980 | 3,270 | 7,000 | 3,435 | 8,740 | 3,555 | 9,050 |
| 1/2 01 #4 | 6 | 4,640 | 9,855 | 4,740 | 10,890 | 4,910 | 12,495 | 5,155 | 13,115 | 5,335 | 13,580 |
| | 10 | 7,730 | 19,680 | 7,905 | 20,115 | 8,180 | 20,825 | 8,590 | 21,860 | 8,890 | 22,630 |
| | 3-1/8 | 2,345 | 3,740 | 2,395 | 4,135 | 2,480 | 4,840 | 2,605 | 6,045 | 2,695 | 7,075 |
| 5/8 or #5 | 5 | 3,750 | 8,270 | 3,835 | 9,140 | 3,970 | 10,700 | 4,165 | 12,960 | 4,315 | 13,415 |
| J/O 01 #J | 7-1/2 | 5,625 | 15,060 | 5,750 | 16,645 | 5,955 | 18,520 | 6,250 | 19,445 | 6,470 | 20,125 |
| | 12-1/2 | 9,375 | 29,175 | 9,585 | 29,820 | 9,920 | 30,865 | 10,415 | 32,405 | 10,780 | 33,540 |
| | 3-1/2 | 2,945 | 4,715 | 3,005 | 5,210 | 3,100 | 6,100 | 3,245 | 7,615 | 3,350 | 8,915 |
| 3/4 or #6 | 6 | 5,190 | 11,475 | 5,305 | 12,680 | 5,490 | 14,845 | 5,765 | 17,940 | 5,970 | 18,570 |
| 5/4 01 #0 | 9 | 7,785 | 20,895 | 7,960 | 23,095 | 8,240 | 25,630 | 8,650 | 26,910 | 8,955 | 27,855 |
| | 15 | 12,980 | 40,375 | 13,265 | 41,270 | 13,730 | 42,720 | 14,415 | 44,850 | 14,920 | 46,425 |
| | 3-1/2 | 2,920 | 4,870 | 2,980 | 5,380 | 3,080 | 6,300 | 3,220 | 7,865 | 3,325 | 9,210 |
| 7/8 or #7 | 7 | 6,850 | 14,275 | 7,000 | 15,775 | 7,245 | 18,465 | 7,610 | 23,060 | 7,875 | 24,500 |
| //o UI #/ | 10-1/2 | 10,275 | 25,995 | 10,500 | 28,730 | 10,870 | 33,635 | 11,410 | 35,505 | 11,815 | 36,750 |
| | 17-1/2 | 17,125 | 53,270 | 17,500 | 54,450 | 18,115 | 56,365 | 19,020 | 59,175 | 19,690 | 61,250 |
| | 4 | 3,720 | 6,115 | 3,805 | 6,775 | 3,930 | 7,935 | 4,110 | 9,905 | 4,245 | 11,600 |
| 1 or #8 | 8 | 8,710 | 17,245 | 8,900 | 19,060 | 9,215 | 22,315 | 9,675 | 27,865 | 10,015 | 31,150 |
| 101#8 | 12 | 13,065 | 31,410 | 13,350 | 34,710 | 13,820 | 40,640 | 14,510 | 45,140 | 15,020 | 46,725 |
| | 20 | 21,770 | 66,930 | 22,255 | 69,230 | 23,035 | 71,665 | 24,185 | 75,235 | 25,030 | 77,880 |

🔲 - Concrete Breakout Strength 🔲 - Bond Strength/Pryout Strength

1. Tabular values are provided for illustration and are applicable for single anchors installed in uncracked normal-weight concrete with minimum slab thickness,

 $h_a=h_{\text{min}}\text{, and with the following conditions:}$

- cat is greater than or equal to the critical edge distance, cac

- Ca2 is greater than or equal to 1.5 times Ca1.

 Calculations were performed according to ACI 318-11 Appendix D and ICC-ES AC308. The load level corresponding to the failure mode listed [Concrete breakout strength, bond strength/ pryout strength] must be checked against the tabulated steel strength of the corresponding threaded rod or rebar size and type, the lowest load level controls.

3. Strength reduction factors (ϕ) for concrete breakout strength are based on ACI 318 Section 9.2 for load combinations. Condition B was assumed.

4. Strength reduction factors (φ) for bond strength are determined from reliability testing and qualification in accordance with ICC-ES AC308 and are tabulated in this product information and in ESR-2583.

5. Tabular values are permitted for static loads only, seismic loading is not considered with these tables. Periodic special inspection must be performed where required by code, see ESR-2583 for applicable information.

6. For anchors subjected to tension resulting from sustained loading a supplemental check must be performed according to ACI 318-11 D.4.1.2.

7. For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-11 Appendix D.

 Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths, please see ACI 318-11 Appendix D, ICC-ES AC308 and information included in this product supplement. For other design conditions including seismic considerations please see ACI 318-11 Appendix D and ICC-ES AC308 and ESR-2583.

9. Long term concrete temperatures are roughly constant over significant periods of time. Short-term elevated temperatures are those that occur over brief intervals, e.g. as a result of diurnal cycling.





Tension Design of Steel Elements (Steel Strength)

| | Steel Elements - Threaded Rod and Reinforcing Bar | | | | | | | | | |
|--|---|---------------------------|---|--|---|--|--------------------------------|--------------------------------|--------------------------------|--------------------------------|
| Nominal Rod/Rebar Size (in. or No.) | ASTM A36 and ASTM F1554 Grade 36 | ASTM F1554 Grade 55 | ASTM A193 Grade B7 and ASTM F1554 Grade 105 | ASTM F593 CW Stainless (Types 304 and 316) | ASTM A193 Grade B8/ B8M, Class 1 Stainless (Types 304 and 316) | ASTM A193 Grade B8/ B8M2, Class 2B Stainless (Types 304 and 316) | ASTM A615 Grade 75 Rebar | ASTM A615 Grade 60 Rebar | ASTM A706 Grade 60 Rebar | ASTM A615 Grade 40 Rebar |
| | ØNsa Tension (Ibs.) | ØNsa Tension (lbs.) | ØNsa Tension (Ibs.) | ØNsa Tension (lbs.) | ØNsa Tension (lbs.) | ØNsa Tension (lbs.) | ØNsa Tension (Ibs.) | ØN₅a Tension (lbs.) | ØNsa Tension (lbs.) | ØN Tension (Ibs.) |
| 3/8 or #3 | 3,370 | 4,360 | 7,265 | 5,040 | 3,315 | 5,525 | 7,150 | 7,425 | 6,600 | 4,950 |
| 1/2 or #4 | 6,175 | 7,980 | 13,300 | 9,225 | 6,070 | 10,110 | 13,000 | 13,500 | 12,000 | 9,000 |
| 5/8 or #5 | 9,835 | 12,715 | 21,190 | 14,690 | 9,660 | 16,105 | 20,150 | 20,925 | 18,600 | 13,950 |
| 3/4 or #6 | 14,550 | 18,815 | 31,360 | 18,480 | 14,300 | 23,830 | 28,600 | 29,700 | 26,400 | 19,800 |
| 7/8 or #7 | 20,085 | 25,970 | 43,285 | 25,510 | 19,735 | 32,895 | 39,000 | 40,500 | 36,000 | - |
| 1 or #8 | 26,350 | 34,070 | 56,785 | 33,465 | 25,895 | 43,160 | 51,350 | 53,325 | 47,400 | - |
| #9 | | | | | | | 65,000 | 67,500 | 60,000 | - |
| 1-1/4 or #10 | 42,160 | 54,510 | 9,100 | 53,540 | 41,430 | 69,050 | 82,550 | 85,725 | 76,200 | - |
| - Steel Strength | | | | | | | | | | |

Shear Design of Steel Elements (Steel Strength)

| | Steel Elements - Threaded Rod and Reinforcing Bar | | | | | | | | | |
|--|---|---------------------------|---|--|---|--|--------------------------------|--------------------------------|--------------------------------|--------------------------------|
| Nominal Rod/Rebar Size (in. or No.) | ASTM A36 and ASTM F1554 Grade 36 | ASTM F1554 Grade 55 | ASTM A193 Grade B7 and ASTM F1554 Grade 105 | ASTM F593 CW Stainless (Types 304 and 316) | ASTM A193 Grade B8/ B8M, Class 1 Stainless (Types 304 and 316) | ASTM A193 Grade B8/ B8M2, Class 2B Stainless (Types 304 and 316) | ASTM A615 Grade 75 Rebar | ASTM A615 Grade 60 Rebar | ASTM A706 Grade 60 Rebar | ASTM A615 Grade 40 Rebar |
| | ØV₃ Tension (lbs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (Ibs.) | ØVsa Tension (lbs.) | ØVsa Tension (Ibs.) |
| 3/8 or #3 | 1,755 | 2,265 | 3,775 | 2,790 | 1,725 | 2,870 | 3,960 | 3,860 | 3,430 | 2,575 |
| 1/2 or #4 | 3,210 | 4,150 | 6,915 | 5,110 | 3,155 | 5,255 | 7,200 | 7,020 | 6,240 | 4,680 |
| 5/8 or #5 | 5,115 | 6,610 | 11,020 | 8,135 | 5,025 | 8,375 | 11,160 | 10,880 | 9,670 | 7,255 |
| 3/4 or #6 | 7,565 | 9,785 | 16,305 | 10,235 | 7,435 | 12,390 | 15,840 | 15,445 | 13,730 | 10,295 |
| 7/8 or #7 | 10,445 | 13,505 | 22,505 | 14,130 | 10,265 | 17,105 | 21,600 | 21,060 | 18,720 | - |
| 1 or #8 | 13,700 | 17,715 | 29,525 | 18,535 | 13,465 | 22,445 | 28,440 | 27,730 | 24,650 | - |
| #9 | | | | | | | 36,000 | 35,100 | 31,200 | - |
| 1-1/4 or #10 | 21,920 | 28,345 | 4,735 | 29,655 | 21,545 | 35,905 | 45,720 | 44,575 | 39,625 | - |
| - Steel Strength | | | | | | | | | | |

PE1000+® ADHESIVE ANCHORS Epoxy Injection Adhesive Anchoring System

INSTALLATION INSTRUCTIONS (SOLID BASE MATERIALS)

ADHESIVE ANCHORS

PE1000+® Epoxy Injection Adhesive Anchoring System

PERMISSIBLE INSTALLATION CONDITIONS Dry Concrete: cured concrete that, at the time of adhesive anchor installation, has not been exposed to water for the preceding 14 days. Water-Saturated Concrete (wet): cured concrete that, at the time of adhesive anchor installation, has been exposed to water over a sufficient length of time to have the maximum possible amount of absorbed water into the concrete pore structure to a depth equal to the anchor embedment depth.

Water-Filled Holes (flooded): cured concrete that is water-saturated and where the drilled hole contains standing water at the time of anchor installation.

HAMMER DRILLING

| DRILLING | |
|---|--|
| | 1- Drill a hole into the base material with a rotary hammer drill tool to the size and embedment required by the selected steel hardware element (reference installation specifications for threaded rod and reinforcing bar). The tolerances of the carbide drill bit must meet ANSI Standard B212.15. |
| · · · · · · · · · · · · · · · · · · · | • Precaution: Use suitable eye and skin protection. Avoid inhalation of dust during drilling and/or removal. |
| | • Note! In case of standing water in the drilled hole (flooded hole condition), all the water has to be removed from the hole (e.g. vacuum, compressed air, etc.) prior to cleaning. |
| HOLE CLEANING | G (BLOW 4X, BRUSH 4X, BLOW 4X) |
| | 2a- Starting from the bottom or back of the drilled anchor hole, blow the hole clean using a compressed air nozzle (min. 90 psi) or a hand pump (supplied by Powers Fasteners) a minimum of four times (4x). |
| <u>ک</u> | • Use a compressed air nozzle (min. 90 psi) or a hand pump (min. volume 25 fl. oz.) for anchor rod 3/8" to 3/4" diameter or reinforcing bar (rebar) sizes #3 to #6. |
| | • Use a compressed air nozzle (min. 90 psi) for anchor rod 7/8" to 1-1/4" diameter and rebar sizes #7 to #10. A hand pump shall not be used with these anchor sizes. |
| | 2b- Determine wire brush diameter (reference hole cleaning equipment selection table) and attach the brush with adaptor to a rotary drill tool or battery screw gun. Brush the hole with the selected wire brush a minimum of four times (4x). A brush extension (supplied by Powers Fasteners, Cat. #08282) should be used for holes drilled deeper than the listed brush length. |
| 4X | • The wire brush diameter must be checked periodically during use. The brush must be replaced if it becomes worn (less than D _{min} , reference hole cleaning equipment selection table) or does not come into contact with the sides of the drilled hole. |
| ► | 2c- Finally, blow the hole clean again a minimum of four times (4x). |
| () () () () () () () () () () () () () (| • Use a compressed air nozzle (min. 90 psi) or a hand pump (min. volume 25 fl. oz.) for anchor rod 3/8" to 3/4" diameter or reinforcing bar (rebar) sizes #3 to #6. |

- (rebar) sizes #3 to #6.
- Use a compressed air nozzle (min. 90 psi) for anchor rod 7/8" to 1-1/4" diameter and rebar sizes #7 to #10. A hand pump shall not be used with these anchor sizes.

When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material.

NEXT GO TO STEP 3.

CORE DRILLING

DRILL

| LING | |
|------|--|
| | 1- Drill a hole into the base material with a core drill tool to the size and embedment required by the selected steel hardware element (reference installation table). The tolerances of the carbide drill bit must meet ANSI Standard B212.15. |
| | Precaution: Use suitable eye and skin protection. Avoid inhalation of dust during drilling and/or removal. |

HOLE CLEANING (RINSE, BRUSH 4X, RINSE, BLOW 4X, BRUSH 4X, BLOW 4X)



v

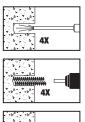
4X

2a- Starting from the bottom or back of the drilled anchor hole, rinse/flush the hole clean with water (water line pressure) until clear water comes out. 2b- Determine brush diameter (see installation table) for drilled hole and attach the brush with adaptor to a rotary drill tool or battery screw

- gun. Brush the hole with the selected wire brush a minimum of four times (4x).
- A brush extension (supplied by Powers Fasteners) must be used for holes drilled deeper than the listed brush length. The wire brush diameter must be checked periodically during use $(\phi_{rush} > D_{min})$ see hole cleaning equipment table). The brush should resist insertion into the drilled hole, if not the brush is small and must be replaced with the proper brush diameter. 2c- Repeat Step 2a again by rinse/flush the hole clean with water.

Following this remove all standing water completely (e.g. vacuum, compressed air, etc.) prior to further cleaning. To attain a dried borehole a

RINSE



4X

2d- Starting from the bottom or back of the drilled anchor hole, blow the hole clean (free of noticeable dust) a minimum if four times (4x). Use a compressed air nozzle (min. 90 psi) for all sizes of anchor rod and reinforcing bar (rebar)

2e- Repeat Step 2b again by brushing the hole with a wire brush a minimum of four times (4x).

2f- Repeat Step 2d again by blowing the hole clean a minimum of four times (4x).

Powers compressed air nozzle is recommended.

When finished the hole should be clean and free of dust, debris, ice, grease, oil or other foreign material.

NEXT GO TO STEP 3.



INSTALLATION INSTRUCTIONS (SOLID BASE MATERIALS)

PREPARING



- 3- Check adhesive expiration date on cartridge label. Do not use expired product. Review Safety Data Sheet (SDS) before use. Cartridge temperature must be between 41°F 95°F (5°C 35°C) when in use; Consideration should be given to the reduced gel time of the adhesive in warm temperatures.
- Attach a supplied mixing nozzle to the cartridge. Do not modify the mixer in any way and make sure the mixing element is inside the nozzle. Load the cartridge into the correct dispensing tool.
- A new mixing nozzle must be used for every working interruption longer than the published working times (reference gel time and curing time table) as well as for new cartridges.
- Note: Always use a new mixing nozzle with new cartridge of adhesive and also for all work interruptions exceeding the published gel (working) time of the adhesive.



- 4- Prior to inserting the anchor rod or rebar into the filled bore hole, the position of the embedment depth has to be marked on the anchor. Verify anchor element is straight and free of surface damage.
- J 3X
- 5- For new cartridges and nozzles: prior to dispensing into the anchor hole, squeeze out separately a minimum three full strokes of the mixed
- adhesive. Discard non-uniform adhesives until the adhesive is a consistent **RED** color.
- Review and note the published working and cure times (reference gel time and curing time table) prior to injection of the mixed adhesive
 into the cleaned anchor hole.

INSTALLATION



6- Fill the cleaned hole approximately two-thirds full with mixed adhesive starting from the bottom or back of the anchor hole. Slowly withdraw the mixing nozzle as the hole fills to avoid creating air pockets or voids. For embedment depth greater than 8 inches an extension nozzle (3/8" dia.) must be used with the mixing nozzle.



Piston plugs (see adhesive piston plug table) must be used with and attached to the mixing nozzle and extension tube for horizontal and
overhead installations with anchor rod from 3/4" to 1-1/4" diameter and rebar size #6 to #10. Insert piston plug to the back of the drilled
hole and inject as described in the method above. During installation the piston plug will be naturally extruded from the drilled hole by the
adhesive pressure.



Attention! Do not install anchors overhead without proper training and installation hardware provided by the Powers Fasteners. Contact
Powers for details prior to use.



7- The anchor should be free of dirt, grease, oil or other foreign material. Push clean threaded rod or reinforcing bar into the anchor hole while turning slightly to ensure positive distribution of the adhesive until the embedment depth is reached. Air pockets are present when the threaded rod or rebar springs or air pockets burst during installation. In case of air pockets: remove rod or rebar, let the adhesive harden, re-drill the hole and repeat the complete installation.



8- Be sure that the anchor is fully seated at the bottom of the hole and that some adhesive has flowed from the hole and all around the top of the anchor. If there is not enough adhesive in the hole, the installation must be repeated. For overhead applications the anchor must be secured from moving/falling during the cure time(e.g. wedges). Minor adjustments to the anchor may be performed during the gel time but the anchor shall not be moved after the final placement and during cure.

CURING AND LOADING



- 9- Allow the adhesive anchor to cure to the specified full curing time prior to applying any load (reference gel time and curing time table).
- Do not disturb, torque or load the anchor until it is fully cured.



- **10-** After full curing of the adhesive anchor, a fixture can be installed to the anchor and tightened up to the maximum torque (reference gel time and curing table) by using a calibrated torque wrench.
- Take care not to exceed the maximum torque for the selected anchor.

REFERENCE TABLES FOR INSTALLATION

Gel (working) Time and Curing Table

| Temperature o | f Base Material | Gel (working) Time | Full Curing Time | | | |
|---------------|-----------------|--------------------|------------------|--|--|--|
| °F | °C | Ger (working) Time | Full Curing Time | | | |
| 41 | 5 | 180 minutes | 50 hours | | | |
| 50 | 10 | 120 minutes | 30 hours | | | |
| 68 | 20 | 30 minutes | 10 hours | | | |
| 86 | 30 | 20 minutes | 6 hours | | | |
| 104 | 40 | 20 minutes | 4 hours | | | |

Hole Cleaning Equipment Selection Table for PE1000+

| Threaded rod diameter (inch) | Rebar size (no.) | ANSI drill bit diameter (inch) | Min. brush diameter, D _{min} (inches) | Brush length, L (inches) | Steel wire brush (Cat. #) | Blowout tool | Number of cleaning actions |
|------------------------------------|---------------------|--------------------------------------|--|-----------------------------|---------------------------------|------------------------------|-------------------------------|
| 3/8 | #3 | 7/16 | 0.475 | 6-3/4 | 08284 | | |
| 1/2 | #4 | 9/16 | 0.600 | 6-3/4 | 08285 | Hand-pump | |
| F.(0 | <i>и</i> г | 11/16 | 0.735 | 7-7/8 | 08286 | or compressed air | |
| 5/8 | #5 | 3/4 | 0.790 | 7-7/8 | 08278 | nozzle (min. 90 psi) | 4x blowing |
| 3/4 | #6 | 7/8 | 0.920 | 7-7/8 | 08287 | | 4x brushing |
| 7/8 | #7 | 1 | 1.045 | 11-7/8 | 08288 | | 4x blowing |
| 1 | #8 | 1-1/8 | 1.175 | 11-7/8 | 08289 | Compressed air | |
| 1-1/4 | #9 | 1-3/8 | 1.425 | 11-7/8 | 08290 | nozzle only (min. 90 psi) | |
| - | #10 | 1-1/2 | 1.550 | 11-7/8 | 08291 | | |

1. An SDS-plus adaptor (Cat. #08283) or Jacobs chuck style adaptor (Cat. #08296) is required to attach a steel wire brush to the drill tool.

2. For installations with 5/8 threaded rod and #5 rebar size, the preferred ANSI drill bit diameter is 3/4-inch. If an 11/16-inch ANSI drill bit is used the user must check before injecting the adhesive to verify that the steel anchor element can be inserted into the cleaned bore hole without resistance.

Adhesive Piston Plugs

| Threaded Rod Diameter (inch) | Rebar Size (no.) | ANSI Drill Bit Diameter (inch) | Plug Size (inch) | Plastic Plug (Cat. #) | Horizontal and overhead installations |
|------------------------------------|------------------------|--------------------------------------|------------------------|-----------------------------|--|
| 3/4 | #6 | 7/8 | 7/8 | 08300 | |
| 7/8 | #7 | 1 | 1 | 08301 | |
| 1 | #8 | 1-1/8 | 1-1/8 | 08303 | |
| 1-1/4 | - | 1-3/8 | 1-3/8 | 08305 | |
| - | #10 | 1-1/2 | 1-1/2 | 08309 | |

A plastic extension tube (3/8" dia., Cat. #08291 or Cat. #08297) must be used with piston plugs.

ORDERING INFORMATION

ORDERING INFORMATION

PE1000+ Cartridge System

| - | Std. Ctn. | Pallet |
|-----------------------------------|-----------------------------------|--|
| PE1000+ 13 fl. oz. dual cartridge | 12 | 540 |
| PE1000+ 20 fl. oz. dual cartridge | 12 | 540 |
| PE1000+ 47 fl. oz. dual cartridge | 12 | 624 |
| | PE1000+ 20 fl. oz. dual cartridge | PE1000+ 20 fl. oz. dual cartridge12PE1000+ 47 fl. oz. dual cartridge12 |



One PE1000+ mixing nozzle is packaged with each cartridge.

PE1000+ mixing nozzles must be used to ensure complete and proper mixing of the adhesive.

Extra Mixing Nozzles

| | - | | , |
|---------|--|-----------|--------|
| Cat No. | Description | Std. Ctn. | Pallet |
| 08294 | Extra mixing nozzle (with an 8" extension) for PE1000+ | 2 | 24 |
| 08281 | Mixing nozzle extension, 8" long | 2 | 24 |
| 08297 | Mixing nozzle extension, 20" long | 1 | 12 |

Dispensing Tools for Injection Adhesive

| Cat No. | Description | Std. Ctn. | Std. Carton | |
|---------|---------------------------------------|-----------|----------------|--|
| 08298 | 13 fl. oz. and 20 fl. oz. Manual Tool | 1 | 6 | |
| 08497SD | 20 fl. oz. Pneumatic tool | 1 | - | |
| 08275 | 47 fl. oz. Pneumatic tool | 1 | - | |

Hole Cleaning Tools and Accessories

| Cat No. | Description | Std. Box |
|---------|---|----------|
| 08284 | Wire brush for 7/16"ANSI hole (3/8" rod or #3 rebar), 6-3/4" length | 1 |
| 08285 | Wire brush for 9/16"ANSI hole (1/2" rod or #4 rebar), 6-3/4" length | 1 |
| 08286 | Wire brush for 11/16"ANSI hole (5/8" rod or #5 rebar), 7-7/8" length | 1 |
| 08278 | Wire brush for 3/4"ANSI hole (5/8" rod or #5 rebar), 7-7/8" length | 1 |
| 08287 | Wire brush for 7/8"ANSI hole (3/4" rod or #6 rebar), 7-7/8" length | 1 |
| 08288 | Wire brush for 1"ANSI hole (7/8" rod or #7 rebar), 11-7/8" length | 1 |
| 08289 | Wire brush for 1-1/8"ANSI hole (1" rod or #8 rebar), 11-7/8" length | 1 |
| 08290 | Wire brush for 1-3/8"ANSI hole (1-1/4" rod or #9 rebar), 11-7/8" length | 1 |
| 08291 | Wire brush for 1-1/2"ANSI hole (#10 rebar), 11-7/8" length | 1 |
| 08283 | SDS-plus adapter for steel brushes | 1 |
| 08296 | Standard drill adapter for steel brushes (e.g. Jacobs Chuck) | 1 |
| 08282 | Steel brush extension, 12" length | 1 |
| 08280 | Hand pump/dust blower (25 fl. oz. cylinder volume) | 1 |
| 08292 | Air compressor nozzle with extension, 18" length | 1 |
| 08465 | Adjustable torque wrench with 1/2" square drive (10 to 150 ftlbs.) | 1 |
| 08466 | Adjustable torque wrench with 1/2" square drive (25 to 250 ftlbs.) | 1 |
| 52073 | Adhesive cleaning kit, includes 4 wire brushes (08284, 08285, 08286, 08287), steel brush extension (08282), SDS-plus adapter (08283), standard drill adapter (08296), hand pump/dust blower (08280), gloves and safety glasses | 1 |



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ADHESIVE ANCHORS